

NOTICE OF PROPOSED DEVELOPMENT

Notice is hereby given that an application has been made for planning approval for the following development:

SITE: 28 Riviera Drive, Carlton

PROPOSED DEVELOPMENT: DWELLING AND OUTBUILDING (SHED)

The relevant plans and documents can be inspected at the Council Offices at 47 Cole Street, Sorell during normal office hours, or the plans may be viewed on Council's website at www.sorell.tas.gov.au until **Tuesday 5th November 2024.**

Any person may make representation in relation to the proposal by letter or electronic mail (<u>sorell.council@sorell.tas.gov.au</u>) addressed to the General Manager. Representations must be received no later than **Tuesday 5th November 2024**.

APPLICANT: Lyne Design

APPLICATION NO: DA 2024 / 248 - 1
DATE: 23 October 2024

Part B: Please note that Part B of this form is publicly exhibited.

Full description of Proposal:	Use:					
	Development:					
	Large or complex proposals should be described in a letter or planning report.					
Design and cons	truction cost of proposal:		\$			
Is all, or some th	e work already constructed:		No: □	Yes: □		
Location of proposed	Street address:					
works:	Suburb:		Posto	code:		
	Certificate of Title(s) Volum	e:		Folio:		
Current Use of Site						
Current Owner/s:	Name(s)					
Is the Property of Register?	on the Tasmanian Heritage	No: □	Yes: □	If yes, please provide written advice from Heritage Tasmania		
Is the proposal to than one stage?	o be carried out in more	No: □	Yes: □	If yes, please clearly describe in plans		
Have any potent been undertaker	ially contaminating uses n on the site?	No: □	Yes: □	If yes, please complete the Additional Information for Non-Residential Use		
Is any vegetation proposed to be removed? No:			Yes: □	If yes, please ensure plans clearly show area to be impacted		
Does the propos	al involve land					
administered or owned by either the Crown No: or Council?			Yes: □	If yes, please complete the Council or Crown land section on page 3		
	If a new or upgraded vehicular crossing is required from Council to the front boundary please					
complete the Vehicular Crossing (and Associated Works) application form						
https://www.sor	https://www.sorell.tas.gov.au/services/engineering/					

Sorell Council

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Declarations and acknowledgements

- I/we confirm that the application does not contradict any easement, covenant or restriction specified in the Certificate of Title, Schedule of Easements or Part 5 Agreement for the land.
- I/we consent to Council employees or consultants entering the site and have arranged permission and/or access for Council's representatives to enter the land at any time during normal business hours.
- I/we authorise the provision of a copy of any documents relating to this application to any person for the purposes of assessment or public consultation and have permission of the copyright owner for such copies.
- I/we declare that, in accordance with s52(1) of the Land Use Planning and Approvals Act 1993, that I have notified the owner(s) of the intention to make this application.
- I/we declare that the information in this application is true and correct.

Details of how the Council manages personal information and how you can request access or corrections to it is outlined in Council's Privacy Policy available on the Council website.

- I/we acknowledge that the documentation submitted in support of my application will become a public record held by Council and may be reproduced by Council in both electronic and hard copy format in order to facilitate the assessment process, for display purposes during public exhibition, and to fulfil its statutory obligations. I further acknowledge that following determination of my application, Council will store documentation relating to my application in electronic format only.
- Where the General Manager's consent is also required under s.14 of the *Urban Drainage Act 2013*, by making this application I/we also apply for that consent.

Applicant Signature:	Signature: Date:
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Crown or General Manager Land Owner Consent

If the land that is the subject of this application is owned or administered by either the Crown or Sorell Council, the consent of the relevant Minister or the Council General Manager whichever is applicable, must be included here. This consent should be completed and signed by either the General Manager, the Minister, or a delegate (as specified in s52 (1D-1G) of the *Land Use Planning and Approvals Act 1993*).

Please note:

- If General Manager consent if required, please first complete the General Manager consent application form available on our website www.sorell.tas.gov.au
- If the application involves Crown land you will also need a letter of consent.
- Any consent is for the purposes of making this application only and is not consent to undertaken work or take any other action with respect to the proposed use or development.

1		being responsible for the
administration of land at		Sorell Council
declare that I have given permiss	Development Application: Development Application - 28 Riviera Drive, Carlton - P1.pdf Plans Reference:P1 Date Received:11/10/2024	
Signature of General Manager, Minister or Delegate:	Signature:	Date:

GEOTECH 24-032

ROCK SOLID GEOTECHNICS PTY LTD

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Orielton

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8/3/2024

Geotechnical Assessment / Classification for Proposed Residential Development

28 Riviera Drive, Carlton.

CONTENTS:

Olivia & Dan Maranesi

CONTENTS

SUMMARY
INVESTIGATION
CONDITIONS OF INVESTIGATION
Olivia_maranesi@hotmail.com

2
2
4

APPENDIX 1

APPENDIX 2

APPENDIX 2

APPENDIX 3

APPENDIX 3

APPENDIX 4

APPENDIX 4

APPENDIX 4

APPENDIX 5

APPENDIX 5

Site Plan

Certificate of Others (Building) – Form 55

CSIRO 'Guide to home-owners on foundation maintenance and footing performance'

APPENDIX 3

APPENDIX 4

Forms 35s & 55s (Wastewater)

Wastewater Loading Certificate



Development Application: Development Application - 28 Riviera Drive, Carlton - P1.pdf

SUMMARY

A residential development is proposed by Olivia & Dan Maranesi at 28 Riviera Drive, Carlton (Figure 1). The site is underlain by deep sand over Triassic sandstone bedrock at variable depths.

The site is classified as Class 'S' in accordance with AS2870. It is recommended to found the residence on the underlying sandstone bedrock.

Suitable upslope site drainage should be installed prior to the commencement of construction.

The following Wind Load Classifications (AS4055-2012: Wind Loads for Housing) are appropriate.

•	Terrain Category Classification	TC2.5	Terrain with a few obstructions
•	Shielding Classification	PS	Partial Shielding
•	Topographic Classification	T2	
•	Wind Load Classification	N3	

INVESTIGATION

The Tasmanian Geological Survey 1:50000 Geological Atlas 'Sorell' indicates that the site is underlain by Quaternary aged windblown sands over Triassic sandstone.

A site investigation was completed on Wednesday 5 March, 2024. This included the augering of four test holes to assess the site for foundation conditions, and onsite wastewater and stormwater suitability (4WD mounted SAMPLA25 mechanical auger with 100mm diameter solid flight augers). The locations of the holes are marked on Figure 1.

The block lies on the western side of Riviera Drive (Plate 1). The site is covered in grass, and minor shrubs, and is devoid of trees. The block is undulating in profile. The proposed house site slopes to the south / southeast at between 6 and 8 degrees. The profile encountered in Test Hole #2 consisted of:

0.00 - 0.25m	SAND: fine grained, light grey, rootlets - TOPSOIL
0.25 – 1.20m	SAND: fine grained, grey, dry
1.20 – 1.45m	sandy CLAY: medium plasticity, olive brown, 35% fine to medium grained sand, moist
1.45m+	Mechanical auger refusal on sandstone bedrock 1.45m depth.

Test Hole #1 (Plate 2) encountered a similar profile but with sandstone bedrock (auger refusal) encountered at 1.15m depth.

Groundwater was not encountered in any of the test holes.



Plate 1 – Looking up-slope to the north from Riviera Drive.



Plate 2 – Looking across-slope to the northeast (Test Hole #1).





CONDITIONS OF INVESTIGATION

This report remains the property of Rock Solid Geotechnics Pty. Ltd. (RSG). It must not be reproduced in part or full, or used for any other purpose without written permission of this company. The investigations have been conducted, & the report prepared, for the sole use of the client or agent mentioned on the cover page. Where the report is to be used for any other purpose RSG accepts no responsibility for such other use. The Forms 55 and 35 are not transferable to another body without consultation (reissue) from RSG. The information in this report is current and suitable for use for a period of two years from the date of production of the report, after which time it cannot be used for Building or Development Application.

This report should not be used for submission for Building or Development Application until RSG has been paid in full for its production. RSG accepts no liability for the contents of this report until full payment has been received.

The results & interpretation of conditions presented in this report are current at the time of the investigation only. The investigation has been conducted in accordance with the specific client's requirements &/or with their servants or agent's instructions.

This report contains observations & interpretations based often on limited subsurface evaluation. Where interpretative information or evaluation has been reported, this information has been identified accordingly & is presented based on professional judgement. RSG does not accept responsibility for variations between interpreted conditions & those that may be subsequently revealed by whatever means.

Due to the possibility of variation in subsurface conditions & materials, the characteristics of materials can vary between sample & observation sites. RSG takes no responsibility for changed or unexpected variations in ground conditions that may affect any aspect of the project. The classifications in this report are based on samples taken from specific sites. The information is not transferable to different sites, no matter how close (ie. if the development site is moved from the original assessment site an additional assessment will be required). It is recommended to notify the author should it be revealed that the sub-surface conditions differ from those presented in this report, so additional assessment & advice may be provided.

Investigations are conducted to standards outlined in Australian Standards:

AS1726-1993:

Geotechnical Site Investigations

AS2870-2011:

Residential Slabs and Footings

AS4055-2012:

Wind Loads for Housing

AS1547-2012:

Onsite Domestic Wastewater Management

& as specified in 'Guidelines for Geotechnical Assessment of Subdivisions and Recommended Code of Practise for Site Classification to AS2870 in Tasmania' - Institute of Engineers, Tasmanian Division.

All new developments should subject to strict site maintenance. Attention is drawn to the enclosed information reproduced with the permission from Standards Australia:

CSIRO Information Sheet No. BTF18 – 'Guide to home-owners on foundation maintenance & footing performance'.



Development Application: Development Application - 28 Riviera Drive, Carlton - P1.pdf

Any assessment that has included an onsite wastewater system design will require a further site visit / inspection once the system has been installed. After the inspection to verify that the system has been installed as per RSG's design a statement will be provided. An additional fee applies for the site visit & issuing the certificate.

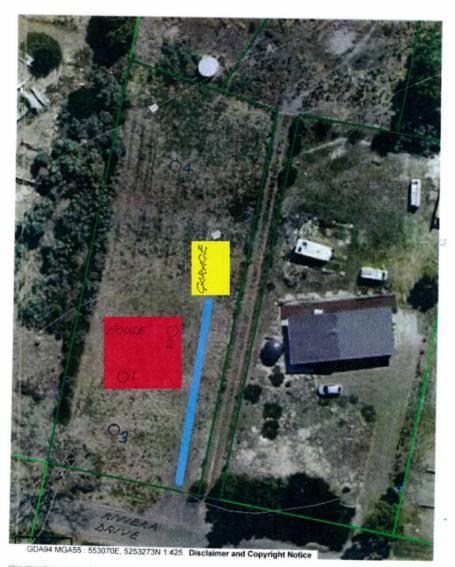
RSG is not responsible for the correct installation of wastewater systems. Any wastewater installation is the sole responsibility of the owner/agent and certified plumber. Any variation to the wastewater design must be approved by RSG, and an amended Special Plumbing Permit obtained from the relevant council. The registered plumber must obtain a copy and carefully follow the details in the council issued Special Plumbing Permit. A "Certificate of Completion" will be based on surface visual inspection only, to verify the location of the system. All underground plumbing works are the responsibility of the certified plumber.

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PETER HOFTO

ROCK SOLID GEOTECHNICS PTY LTD

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https://reaps.theres.tax.gov.au/reimsp/app/list/msp

Page 1 of 1



Development Application: Development Application - 28 Riviera Drive, Carlton - P1.pdf

CERTIFICATE OF QUALIFIED PERSON – ASSESSABLE ITEM

Section 321

To:	To: Olivia & Dan Maranesi			Owner /Agent			
	olivia_maranesi@hotmail.com		Address	Form 55			
						 Suburb/postcode	
Qualified perso	on details:						
Qualified person:	Peter Hofto - Rock Solid Geotec	hnics P/L					
Address:	163 Orielton Road					Phone No:	04179607
	Orielton		7	172		Fax No:	
Licence No:		Email add	ress	pe	eter@	rocksolidgeotech	nnics.com.au
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Address:	28 Riviera Drive, Carlton						Lot No:
						Certificate of t	itle No:
The assessable item related to this certificate:	Geotechnical Assessment Sorell Council Development Application: Development Application - 28 Riviera Drive, Carlton - P1.pdf Plans Reference:P1 Date Received:11/10/2024			(description of the assessable item being certified) Assessable item includes – - a material; - a design - a form of construction - a document - testing of a component, building system or plumbing system - an inspection, or assessment, performed			
Certificate detai	ils:						
Certificate type:	Geotechnical Assessment			Sch	edule ermina ilified F	n from Column 1 or 1 of the Director's tion – Certificates b Persons for Assessa	D y

This certificate is in relation to the above assessable items, at any stage, as part of – (tick one)

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OR

C a building, temporary structure or plumbing installation

Documents:			
Relevant calculations:	AS2870 AS4055		
References:	44.54		
-17	Substance of Certificate: (w	hat it is that is being certified)	
\$ 1			7
	Scope and/or	Limitations	
I certify the matters	described in this certificate.	Certificate No:	Date:
Qualified person:	Signed:	GEOTECH 24-032	8/3/2024
		Sorell Council Development Application - 28 Riviera Drive, Carlton - P1.pdf	
		Plans Reference:P1 Date Received:11/10/2024	

Foundation Maintenance and Footing Performance: A Homeowner's Guide



BTF 18 replaces Information Sheet 10/91

Buildings can and often do move. This movement can be up, down, lateral or rotational. The fundamental cause of movement in buildings can usually be related to one or more problems in the foundation soil. It is important for the homeowner to identify the soil type in order to ascertain the measures that should be put in place in order to ensure that problems in the foundation soil can be prevented, thus protecting against building movement.

This Building Technology File is designed to identify causes of soil-related building movement, and to suggest methods of prevention of resultant cracking in buildings.

Development Application: Development Application - 28 Riviera Drive, Carlton - P1.pdf

Soil Types

The types of soils usually present under the topsoil in land zoned for residential buildings can be split into two approximate groups granular and clay. Quite often, foundation soil is a mixture of both types. The general problems associated with soils having granular content are usually caused by erosion. Clay soils are subject to saturation and swell/shrink problems.

Classifications for a given area can generally be obtained by application to the local authority, but these are sometimes unreliable and if there is doubt, a geotechnical report should be commissioned. As most buildings suffering movement problems are founded on clay soils, there is an emphasis on classification of soils according to the amount of swell and shrinkage they experience with variations of water content. The table below is Table 2.1 from AS 2870, the Residential Slab and Footing Code.

Causes of Movement

Settlement due to construction

There are two types of settlement that occur as a result of construction:

- · Immediate settlement occurs when a building is first placed on its foundation soil, as a result of compaction of the soil under the weight of the structure. The cohesive quality of clay soil mitigates against this, but granular (particularly sandy) soil is susceptible.
- Consolidation settlement is a feature of clay soil and may take place because of the expulsion of moisture from the soil or because of the soil's lack of resistance to local compressive or shear stresses. This will usually take place during the first few months after construction, but has been known to take many years in exceptional cases.

These problems are the province of the builder and should be taken into consideration as part of the preparation of the site for construction. Building Technology File 19 (BTF 19) deals with these problems.

Erosion

All soils are prone to crosion, but sandy soil is particularly susceptible to being washed away. Even clay with a sand component of say 10% or more can suffer from erosion.

Plans Reference:P1 Date Received:11/10/2024

Saturation

This is particularly a problem in clay soils. Saturation creates a boglike suspension of the soil that causes it to lose virtually all of its bearing capacity. To a lesser degree, sand is affected by saturation because saturated sand may undergo a reduction in volume particularly imported sand fill for bedding and blinding layers. However, this usually occurs as immediate settlement and should normally be the province of the builder.

Seasonal swelling and shrinkage of soil

All clays react to the presence of water by slowly absorbing it, making the soil increase in volume (see table below). The degree of increase varies considerably between different clays, as does the degree of decrease during the subsequent drying out caused by fair weather periods. Because of the low absorption and expulsion rate, this phenomenon will not usually be noticeable unless there are prolonged rainy or dry periods, usually of weeks or months, depending on the land and soil characteristics.

The swelling of soil creates an upward force on the footings of the building, and shrinkage creates subsidence that takes away the support needed by the footing to retain equilibrium.

Shear failure

This phenomenon occurs when the foundation soil does not have sufficient strength to support the weight of the footing. There are two major post-construction causes:

- · Significant load increase.
- Reduction of lateral support of the soil under the footing due to erosion or excavation.
- · In clay soil, shear failure can be caused by saturation of the soil adjacent to or under the footing.

	GENERAL DEFINITIONS OF SITE CLASSES			
Class Foundation				
1	Most sand and rock sites with little or no ground movement from moisture changes			
S	Slightly reactive clay sites with only slight ground movement from moisture changes			
M	Moderately reactive clay or silt sites, which can experience moderate ground movement from moisture changes			
Н	Highly reactive clay sites, which can experience high ground movement from moisture changes			
Е	Extremely reactive sites, which can experience extreme ground movement from moisture changes			
A to P	Filled sites			
P	Sites which include soft soils, such as soft clay or silt or loose sands; landslip; mine subsidence; collapsing soils; soils subject to erosion; reactive sites subject to abnormal moisture conditions or sites which cannot be classified otherwise			

Tree root growth

Trees and shrubs that are allowed to grow in the vicinity of footings can cause foundation soil movement in two ways:

- Roots that grow under footings may increase in cross-sectional size, exerting upward pressure on footings.
- Roots in the vicinity of footings will absorb much of the moisture in the foundation soil, causing shrinkage or subsidence.

Unevenness of Movement

The types of ground movement described above usually occur unevenly throughout the building's foundation soil. Settlement due to construction tends to be uneven because of:

- Differing compaction of foundation soil prior to construction.
- Differing moisture content of foundation soil prior to construction.

Movement due to non-construction causes is usually more uneven still. Erosion can undermine a footing that traverses the flow or can create the conditions for shear failure by eroding soil adjacent to a footing that runs in the same direction as the flow.

Saturation of clay foundation soil may occur where subfloor walls create a dam that makes water pond. It can also occur wherever there is a source of water near footings in clay soil. This leads to a severe reduction in the strength of the soil which may create local shear

Seasonal swelling and shrinkage of clay soil affects the perimeter of the building first, then gradually spreads to the interior. The swelling process will usually begin at the uphill extreme of the building, or on the weather side where the land is flat. Swelling gradually reaches the interior soil as absorption continues. Shrinkage usually begins where the sun's heat is greatest.

Effects of Uneven Soil Movement on Structures

Erosion and saturation

Erosion removes the support from under footings, tending to create subsidence of the part of the structure under which it occurs. Brickwork walls will resist the stress created by this removal of support by bridging the gap or cantilevering until the bricks or the mortar bedding fail. Older masonry has little resistance. Evidence of failure varies according to circumstances and symptoms may include:

- Step cracking in the mortar beds in the body of the wall or above/below openings such as doors or windows.
- · Vertical cracking in the bricks (usually but not necessarily in line with the vertical beds or perpends).

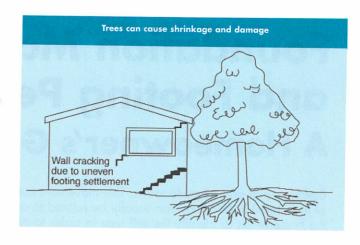
Isolated piers affected by erosion or saturation of foundations will eventually lose contact with the bearers they support and may tilt or fall over. The floors that have lost this support will become bouncy, sometimes rattling ornaments etc.

Seasonal swelling/shrinkage in clay

Swelling foundation soil due to rainy periods first lifts the most exposed extremities of the footing system, then the remainder of the perimeter footings while gradually permeating inside the building footprint to lift internal footings. This swelling first tends to create a dish effect, because the external footings are pushed higher than the internal ones.

The first noticeable symptom may be that the floor appears slightly dished. This is often accompanied by some doors binding on the floor or the door head, together with some cracking of cornice mitres. In buildings with timber flooring supported by bearers and joists, the floor can be bouncy. Externally there may be visible dishing of the hip or ridge lines.

As the moisture absorption process completes its journey to the innermost areas of the building, the internal footings will rise. If the spread of moisture is roughly even, it may be that the symptoms will temporarily disappear, but it is more likely that swelling will be uneven, creating a difference rather than a disappearance in symptoms. In buildings with timber flooring supported by bearers and joists, the isolated piers will rise more easily than the strip footings or piers under walls, creating noticeable doming of flooring.



As the weather pattern changes and the soil begins to dry out, the external footings will be first affected, beginning with the locations where the sun's effect is strongest. This has the effect of lowering the external footings. The doming is accentuated and cracking reduces or disappears where it occurred because of dishing, but other cracks open up. The roof lines may become convex.

Doming and dishing are also affected by weather in other ways. In areas where warm, wet summers and cooler dry winters prevail, water migration tends to be toward the interior and doming will be accentuated, whereas where summers are dry and winters are cold and wet, migration tends to be toward the exterior and the underlying propensity is toward dishing.

Movement caused by tree roots

In general, growing roots will exert an upward pressure on footings, whereas soil subject to drying because of tree or shrub roots will tend to remove support from under footings by inducing shrinkage.

Complications caused by the structure itself

Most forces that the soil causes to be exerted on structures are vertical - i.e. either up or down. However, because these forces are seldom spread evenly around the footings, and because the building resists uneven movement because of its rigidity, forces are exerted from one part of the building to another. The net result of all these forces is usually rotational. This resultant force often complicates the diagnosis because the visible symptoms do not simply reflect the original cause. A common symptom is binding of doors on the vertical member of the frame.

Effects on full masonry structures

Brickwork will resist cracking where it can. It will attempt to span areas that lose support because of subsided foundations or raised points. It is therefore usual to see cracking at weak points, such as openings for windows or doors.

In the event of construction settlement, cracking will usually remain unchanged after the process of settlement has ceased.

With local shear or erosion, cracking will usually continue to develop until the original cause has been remedied, or until the subsidence has completely neutralised the affected portion of footing and the structure has stabilised on other footings that remain effective.

In the case of swell/shrink effects, the brickwork will in some cases return to its original position after completion of a cycle, however it is more likely that the rotational effect will not be exactly reversed, and it is also usual that brickwork will settle in its new position and will resist the forces trying to return it to its original position. This means that in a case where swelling takes place after construction and cracking occurs, the cracking is likely to at least partly remain after the shrink segment of the cycle is complete. Thus, each time the cycle is repeated, the likelihood is that the cracking will become wider until the sections of brickwork become virtually independent.

With repeated cycles, once the cracking is established, if there is no other complication, it is normal for the incidence of cracking to stabilise, as the building has the articulation it needs to cope with the problem. This is by no means always the case, however, and monitoring of cracks in walls and floors should always be treated seriously.

Upheaval caused by growth of tree roots under the root to also the root to also exert lateral forces that attempt to separate sec the second section of the second secti after initial cracking has occurred.

Plans Reference:P1

The normal structural arrangement is that the inner leaf of brickwork in the external walls and at least some of the internal walls (depending on the roof type) comprise the load-bearing structure on which any upper floors, ceilings and the roof are supported. In these cases, it is internally visible cracking that should be the main focus of attention, however there are a few examples of dwellings whose external leaf of masonry plays some supporting role, so this should be checked if there is any doubt. In any case, externally visible cracking is important as a guide to stresses on the structure generally, and it should also be remembered that the external walls must be capable of supporting themselves.

Effects on framed structures

Timber or steel framed buildings are less likely to exhibit cracking due to swell/shrink than masonry buildings because of their flexibility. Also, the doming/dishing effects tend to be lower because of the lighter weight of walls. The main risks to framed buildings are encountered because of the isolated pier footings used under walls. Where erosion or saturation cause a footing to fall away, this can double the span which a wall must bridge. This additional stress can create cracking in wall linings, particularly where there is a weak point in the structure caused by a door or window opening. It is, however, unlikely that framed structures will be so stressed as to suffer serious damage without first exhibiting some or all of the above symptoms for a considerable period. The same warning period should apply in the case of upheaval. It should be noted, however, that where framed buildings are supported by strip footings there is only one leaf of brickwork and therefore the externally visible walls are the supporting structure for the building. In this case, the subfloor masonry walls can be expected to behave as full brickwork walls.

Effects on brick veneer structures

Because the load-bearing structure of a brick veneer building is the frame that makes up the interior leaf of the external walls plus perhaps the internal walls, depending on the type of roof, the building can be expected to behave as a framed structure, except that the external masonry will behave in a similar way to the external leaf of a full masonry structure.

Water Service and Drainage

Where a water service pipe, a sewer or stormwater drainage pipe is in the vicinity of a building, a water leak can cause erosion, swelling or saturation of susceptible soil. Even a minuscule leak can be enough to saturate a clay foundation. A leaking tap near a building can have the same effect. In addition, trenches containing pipes can become watercourses even though backfilled, particularly where broken rubble is used as fill. Water that runs along these trenches can be responsible for serious erosion, interstrata seepage into subfloor areas and saturation.

Pipe leakage and trench water flows also encourage tree and shrub roots to the source of water, complicating and exacerbating the problem.

Poor roof plumbing can result in large volumes of rainwater being concentrated in a small area of soil:

 Incorrect falls in roof guttering may result in overflows, as may gutters blocked with leaves etc.

- · Corroded guttering or downpipes can spill water to ground.
- Downpipes not positively connected to a proper stormwater collection system will direct a concentration of water to soil that is directly adjacent to footings, sometimes causing large-scale problems such as erosion, saturation and migration of water under the building.

Seriousness of Cracking

In general, most cracking found in masonry walls is a cosmetic nuisance only and can be kept in repair or even ignored. The table below is a reproduction of Table C1 of AS 2870.

AS 2870 also publishes figures relating to cracking in concrete floors, however because wall cracking will usually reach the critical point significantly earlier than cracking in slabs, this table is not reproduced here.

Prevention/Cure

Plumbing

Where building movement is caused by water service, roof plumbing, sewer or stormwater failure, the remedy is to repair the problem. It is prudent, however, to consider also rerouting pipes away from the building where possible, and relocating taps to positions where any leakage will not direct water to the building vicinity. Even where gully traps are present, there is sometimes sufficient spill to create erosion or saturation, particularly in modern installations using smaller diameter PVC fixtures. Indeed, some gully traps are not situated directly under the taps that are installed to charge them, with the result that water from the tap may enter the backfilled trench that houses the sewer piping. If the trench has been poorly backfilled, the water will either pond or flow along the bottom of the trench. As these trenches usually run alongside the footings and can be at a similar depth, it is not hard to see how any water that is thus directed into a trench can easily affect the foundation's ability to support footings or even gain entry to the subfloor area.

Ground drainage

In all soils there is the capacity for water to travel on the surface and below it. Surface water flows can be established by inspection during and after heavy or prolonged rain. If necessary, a grated drain system connected to the stormwater collection system is usually an easy solution.

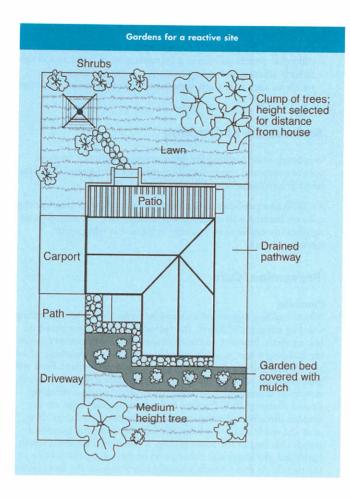
It is, however, sometimes necessary when attempting to prevent water migration that testing be carried out to establish watertable height and subsoil water flows. This subject is referred to in BTF 19 and may properly be regarded as an area for an expert consultant.

Protection of the building perimeter

It is essential to remember that the soil that affects footings extends well beyond the actual building line. Watering of garden plants, shrubs and trees causes some of the most serious water problems.

For this reason, particularly where problems exist or are likely to occur, it is recommended that an apron of paving be installed around as much of the building perimeter as necessary. This paying

evelopment Ap	Council plication: Development Riviera Drive, Carlton - P1.pdf CLASSIFICATION OF DAMAGE WITH	REFERENCE TO WALLS	
Plans Reference Plate Received:		Approximate crack width limit (see Note 3)	Damage
	Hairline cracks	<0.1 mm	0
	Fine cracks which do not need repair	<1 mm	1
	Cracks noticeable but easily filled. Doors and windows stick slightly	<5 mm	2
	Cracks can be repaired and possibly a small amount of wall will need to be replaced. Doors and windows stick. Service pipes can fracture. Weathertightness often impaired	5–15 mm (or a number of cracks 3 mm or more in one group)	3
	Extensive repair work involving breaking-out and replacing sections of walls, especially over doors and windows. Window and door frames distort. Walls lear or bulge noticeably, some loss of bearing in beams. Service pipes disrupted	15–25 mm but also depend on number of cracks	4



should extend outwards a minimum of 900 mm (more in highly reactive soil) and should have a minimum fall away from the building of 1:60. The finished paving should be no less than 100 mm below brick vent bases.

It is prudent to relocate drainage pipes away from this paving, if possible, to avoid complications from future leakage. If this is not practical, earthenware pipes should be replaced by PVC and backfilling should be of the same soil type as the surrounding soil and compacted to the same density.

Except in areas where freezing of water is an issue, it is wise to remove taps in the building area and relocate them well away from the building – preferably not uphill from it (see BTF 19).

It may be desirable to install a grated drain at the outside edge of the paving on the uphill side of the building. If subsoil drainage is needed this can be installed under the surface drain.

Condensation

In buildings with a subfloor void such as where bearers and joists support flooring, insufficient ventilation creates ideal conditions for condensation, particularly where there is little clearance between the floor and the ground. Condensation adds to the moisture already present in the subfloor and significantly slows the process of drying out. Installation of an adequate subfloor ventilation system, either natural or mechanical, is desirable.

Warning: Although this Building Technology File deals with cracking in buildings, it should be said that subfloor moisture can result in the development of other problems, notably:

- Water that is transmitted into masonry, metal or timber building elements causes damage and/or decay to those elements.
- High subfloor humidity and moisture content create an ideal environment for various pests, including termites and spiders.
- Where high moisture levels are transmitted to the flooring and walls, an increase in the dust mite count can ensue within the living areas. Dust mites, as well as dampness in general, can be a health hazard to inhabitants, particularly those who are abnormally susceptible to respiratory ailments.

The garden

The ideal vegetation layout is to have lawn or plants that require only light watering immediately adjacent to the drainage or paving edge, then more demanding plants, shrubs and trees spread out in that order.

Overwatering due to misuse of automatic watering systems is a common cause of saturation and water migration under footings. If it is necessary to use these systems, it is important to remove garden beds to a completely safe distance from buildings.

Existing trees

Where a tree is causing a problem of soil drying or there is the existence or threat of upheaval of footings, if the offending roots are subsidiary and their removal will not significantly damage the tree, they should be severed and a concrete or metal barrier placed vertically in the soil to prevent future root growth in the direction of the building. If it is not possible to remove the relevant roots without damage to the tree, an application to remove the tree should be made to the local authority. A prudent plan is to transplant likely offenders before they become a problem.

Information on trees, plants and shrubs

State departments overseeing agriculture can give information regarding root patterns, volume of water needed and safe distance from buildings of most species. Botanic gardens are also sources of information. For information on plant roots and drains, see Building Technology File 17.

Excavation

Excavation around footings must be properly engineered. Soil supporting footings can only be safely excavated at an angle that allows the soil under the footing to remain stable. This angle is called the angle of repose (or friction) and varies significantly between soil types and conditions. Removal of soil within the angle of repose will cause subsidence.

Remediation

Where erosion has occurred that has washed away soil adjacent to footings, soil of the same classification should be introduced and compacted to the same density. Where footings have been undermined, augmentation or other specialist work may be required. Remediation of footings and foundations is generally the realm of a specialist consultant.

Where isolated footings rise and fall because of swell/shrink effect, the homeowner may be tempted to alleviate floor bounce by filling the gap that has appeared between the bearer and the pier with blocking. The danger here is that when the next swell segment of the cycle occurs, the extra blocking will push the floor up into an accentuated dome and may also cause local shear failure in the soil. If it is necessary to use blocking, it should be by a pair of fine wedges and monitoring should be carried out fortnightly.

This BTF was prepared by John Lewer FAIB, MIAMA, Partner, Construction Diagnosis.

The information in this and other issues in the series was derived from various sources and was believed to be correct when published.

The information is advisory. It is provided in good faith and not claimed to be an exhaustive treatment of the relevant subject.

Further professional advice needs to be obtained before taking any action based on the information provided.

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Sorell Council

Development Application: Development Application - 28 Riviera Drive, Carlton - P1.pdf

APPENDIX 3

ONSITE WASTEWATER ASSESSMENT / SYSTEM DESIGN - 28 Riviera Drive, Carlton

Below find the assessment to determine of the type and size of wastewater treatment system, and the allocation of a Land Application Area (LAA) for a proposed 4-bedroom residence at 28 Riviera Drive, Carlton. This assessment should be read in conjunction with Site & Soil Evaluation Report (GEOTECH 24-032) - enclosed.

The block lies on the western side of Riviera Drive (Plate 1). The site is covered in grass, and minor shrubs, and is devoid of trees. The block is undulating in profile. The site for the proposed wastewater LAA slopes to the south / southeast at 7 degrees.

The profile encountered in Test Hole #3 (Plate 3) consisted of:

0.00 - 0.20m

SAND: fine grained, light grey, rootlets - TOPSOIL

0.20 - 1.55m

SAND: fine grained, grey, dry

1.55m+

Mechanical auger refusal on sandstone bedrock 1.55m depth.

Groundwater was not encountered in any of the test holes.

The site is classified as a Class 1 (SAND) site with an Indicative Permeability of 1.5 m/day. A Design Loading Rate of 30mm/day is appropriate.

It is proposed to install a septic tank discharging to an in-ground Advanced Enviro-Septic (AES) bed, sited on the southern portion of the property. This area should be protected from vehicles as driving over the pipework will likely destroy the system.

There is suitable area available for a reserve LAA if required in the future.

Plate 3 – Looking to the southeast (Test Hole #3).



COMPLIANCE WITH THE 2016 DIRECTOR'S GUIDELINES FOR ONSITE WASTEWATER

Compliance Table	Directors Guidelines for OSWM	
Acceptable Solutions	Performance Criteria	Compliance achieved by
5.1 To ensure sufficient land is available for sustainable onsite wastewater management for buildings.		- 0
A1 A new dwelling must be provided with a LAA that complies with Table 3.	P1 A new dwelling must be provided with a LAA that meets all of the following: a) The LAA is sized in accordance with the requirements of AS/NZS 1547; and b) A risk assessment in accordance with Appendix A of AS/NZS 1547 has been completed that demonstrates that the risk is acceptable.	Complies with A1 50m² of LAA required /bedroom, or 200m² for this development
7. Standards for Wastewater Land Application Areas		
A1 Horizontal separation distance from a building to a LAA must comply with one of the following: a) be no less than 6m; b) be no less than: (i) 3m from an upslope boundary or level building; (ii) If primary treated effluent to be no less	P1 The LAA is located so that the risk of wastewater reducing the bearing capacity of a building's foundations is acceptably low.	Complies with A1 LAA > 3m from upslope residence.



than 4m plus 1m for every degree of average gradient from a downslope building; (iii) If secondary treated effluent and subsurface application, no less than 2m plus 0.25m for every degree of average gradient from a downslope building. A2 Horizontal separation distance from downslope surface water to a LAA must comply with (a) or (b) (a) be no less than 100m; or (b) be no less than the following: (i) if primary treated effluent 15m plus 7m for every degree of average gradient to downslope surface water; or (ii) if secondary treated effluent and subsurface application, 15m plus 2m for every degree of average gradient to down slope surface water.	P2 Horizontal separation distance from downslope surface water to a LAA must comply with all of the following: a) Setbacks must be consistent with AS/NZS 1547 Appendix R; b) A risk assessment in accordance with Appendix A of AS/NZS 1547 has been completed that demonstrates that the risk is acceptable.	Complies with A2 LAA >100m from downslope surface water.
Horizontal separation distance from a property boundary to a LAA must comply with either of the following: (a) be no less than 40m from a property boundary; or (b) be no less than: (i) 1.5m from an upslope or level property boundary; & (ii) If primary treated effluent 2m for every degree of average gradient from a downslope property boundary; or (iii) If secondary treated effluent and subsurface application, 1.5m plus 1m for every degree of average gradient from a downslope property boundary.	Horizontal separation distance from a property boundary to a LAA must comply with all of the following: (a) Setback must be consistent with AS/NZS 1547 Appendix R; and (b) A risk assessment in accordance with Appendix A of AS/NZS 1547 has been completed that demonstrates that the risk is acceptable.	Complies with A3 LAA > 1.5m from upslope and side-slope property boundaries. 8° slope. Setback required from lower slope property boundary 1.5m + (1m x 8°) = 9.5m
Horizontal separation distance from a downslope bore, well or similar water supply to a LAA must be no less than 50m and not be within the zone of influence of the bore whether up or down gradient.	P4 Horizontal separation distance from a downslope bore, well or similar water supply to a LAA must comply with all of the following: (a) Setback must be consistent with AS/NZS 1547 Appendix R; and (b) A risk assessment completed in accordance with Appendix A of AS/NZS 1547 demonstrates that the risk is acceptable.	Complies with A4 No known potable bores in the immediate vicinity.
Vertical separation distance between groundwater & a LAA must be no less than: (a) 1.5m if primary treated effluent; or (b) 0.6m if secondary treated effluent	P5 Vertical separation distance between groundwater and a LAA must comply with the following: (a) Setback must be consistent with AS/NZS 1547 Appendix R; and (b) A risk assessment completed in accordance with Appendix A of AS/NZS 1547 that demonstrates that the risk is acceptable.	Complies with A5 Groundwater not encountered.
Vertical separation distance between a limiting layer & a LAA must be no less than: (a) 1.5m if primary treated effluent; or (b) 0.5m if secondary treated effluent.	P6 Vertical setback must be consistent with AS/NZS1547 Appendix R.	Complies with A6 Limiting layer at 1.55m depth.



ONSITE WASTEWATER SYSTEM DESIGN

A new, 3250 litre (minimum) septic tank will be installed. The effluent leaving the septic tank will gravity feed an Advanced Enviro-Septic (AES) bed. The septic tank should not be fitted with an outlet filter.

The following calculations determine the size of the AES Bed designed to service the 4-bedroom.

4-bedroom residence

6 persons

Tank water

120 litres / person / day

Wastewater Flow Rate

 $6 \times 120 = 720$ litres / day

Design Loading Rate (DLR)

30mm/day

DLR

30 litres / m2 / day

Basal Area of Land Application Area

 $720 / 30 = 24m^2$

The Advanced Enviro Septic (AES) system utilizes a modular distribution layout consisting of pipework laid in "system sand".

This module consists of 2 runs of 4 x 300mm diameter AES pipes, 150mm apart, with 300mm side-wall clearance on each side - total width 1350mm.

Distribution unit length

= AES pipe length + $(0.3m \times 2)$

12m + 0.6m = 12.6m

Width of 4-pipe wide AES unit

= 1.35m

A System Extension is required for this site.

12.6m long x 0.55m wide = $7m^2$

Area of AES

= 12.6m x 1.90m = $\frac{24m^2}{}$

A cutoff drain will not be required (house site drainage should suffice).

The AES system should be installed by a plumber who has been accredited by Chankar Environmental Proprietary Limited to install Advanced Enviro Septic systems, and who has appropriate experience.



Development Application: Development Application - 28 Riviera Drive, Carlton - P1.pdf

Site Preparation

- Rope off the site to prevent damage to the area during other construction activity on the lot.
- Vehicular traffic over the area must be prohibited to avoid compaction.
- Excavate the existing soil surface, parallel with the contour (cross slope) to a depth of 750mm over the selected wastewater land application area.
- Rake/scarify the exposed soil surface.
- Install and connect the septic tank and AES bed in accordance with the AES site instructions (see below) and the
 design plans attached.
- The AES pipe must be laid in a bed of approved "system sand". This is a coarse sand meeting the specifications as listed below.

AES System Sand Specifications

- Percentage Restrictions 35% or less of the total sand may be gravel. 40%-90% of the total sand is to be coarse
 and very coarse sand.
- Gravel Quality Restrictions No gravel is to exceed 9mm in diameter. No gravel is smaller than 2mm in diameter.
- Coarse Sand Quality Restrictions No coarse sand is smaller than 0.5mm in diameter.
- $\bullet~$ Fines Quality Restrictions No more than 2% of the total sand may pass through a 75 $\mu~$ m sieve.

Venting - AES system and septic tank

- Ensure that roof vent comprises a minimum of single 80mm diameter pipe or 2 x 40mm diameter vent pipes.
- Roof vent to be a minimum of 3m above ground vent.
- Venting of the septic tank is to be consistent with NCC Pt 3 Tas F101.2.

Low vent as per AES pipe layout plan (Low vent at end of pipework).





Advanced Enviro-septic Design Calculator V9.0 ©

	AES The World Leader in Passive Solutions ©					
Site Address	28 Riviera Drive, Carlton		State TAS	Post Code	7173	
Client Name	Client Name Olivia Maranesi			Date of Site Visit	5/3/24	
Designers Name	Peter Hofto, Rock Solid Geotechnics Pty Ltd	Designers Ph Number	0417 960 769	Designer Lic (e.gQBCC)	CC6159I	
Lic Plumber	To be announced	Plumber Ph Number		Plumb / Drainer Lic Number	TBA	
Council Area	Sorell	Designers AES Cert Number	1463	Date	8/3/24	

This Calculator is a guide only, receiving soil classification, surface water, water tables and all other site constraints addressed by the qualified designer.

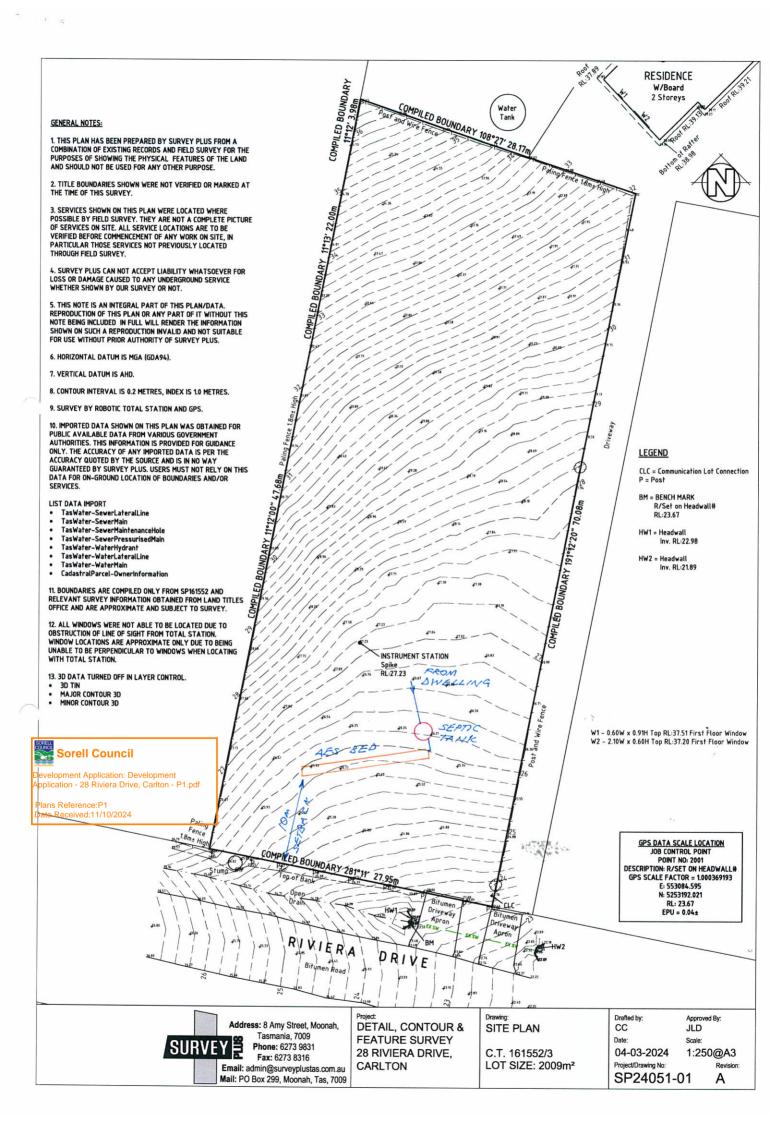
System Designers site and soil calculation data entry		IMPORTANT NOTES
Enter AES L/m loading rate, "30" for ADV Secondary or "38" Secondary	38	>> This design is for a SECONDARY system.
Is this a new installation Y or N	Y	>> Minimun single vent size is 80mm or 2 x 50mm house vents
Number of Bedrooms	4	>> This is not used in ANY Calculation. If not known use N/A or 0.
Number of persons	6	>> A septic tank outlet filter is NOT RECOMMENDED
Daily Design Flow Allowance Litre/Person/Day	120	
Number of rows required to suit site constraints	2	>> The maximum length of a single AES pipe run is 30m or 10 PIPES
Infiltration Soil Category from site/soil evaluation. CATEGORY	1	
Design Loading Rate based on site & soil evaluation DLR (mm/day)	30	
Bore log depth below system Basal area	1.0m	>> Min depth 1.5m. Check water table/restrictive layer
Is this design a GRAVITY system with no outlet filter? Y or N	y	>> GRAVITY. A House Vent & LOW VENT required on this system
PLEASE CHECK YOU HAVE FALL FROM TANK TO AES SYSTEM PIPES		_

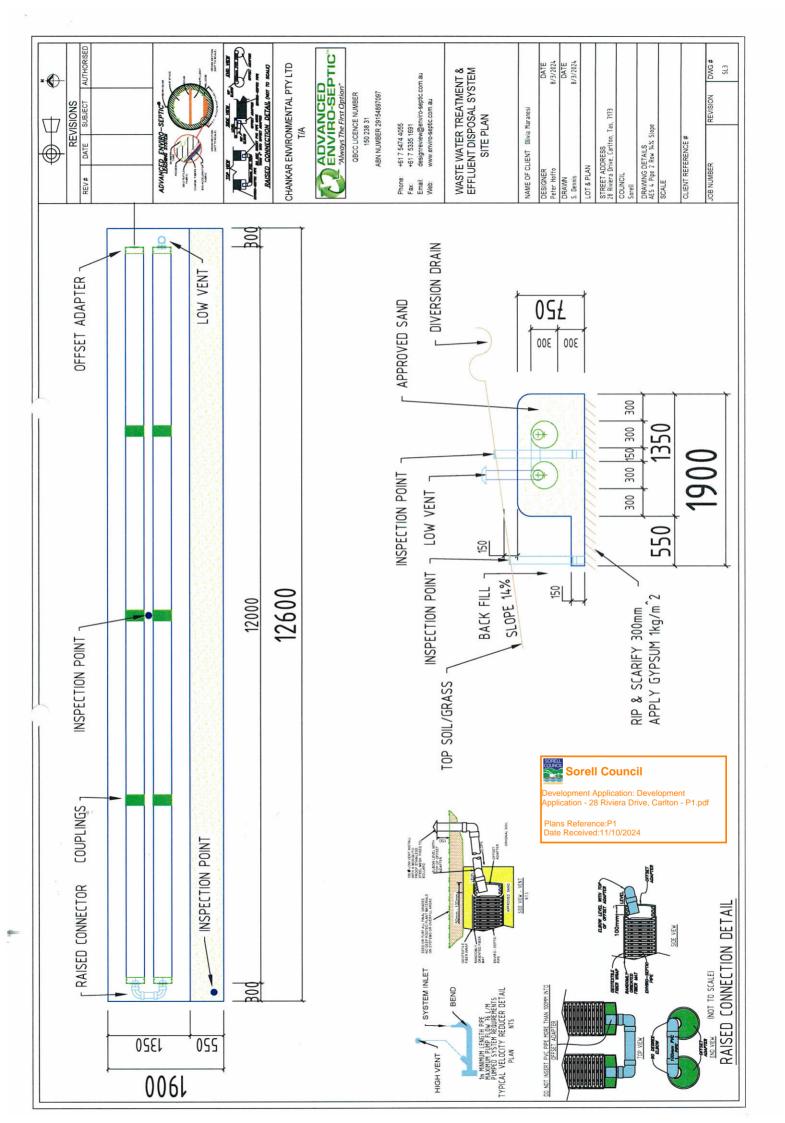
COMMENTS:- " The outcome must be important to everyone."

- Designers need to be familar with special requirements of Local Authorities. ie Minimum falls from Septic tank outlets to Land application areas etc
- Plumbers are reminded to practice good construction techniques as per AS 1547 & as provided on AES installation instructions supplied with components.

	AES System Calculator Outcomes					AES dimension	ons
	Total System load - litres / day (Q).	720	I/d			AES System	System Extension
	Min Length of AES pipe rows to treat loading	9.47	lm		Length:(L)	12.60m	12.60m
	Number of FULL AES Pipe lengths per row	4	Iths		Width:(W)	1.35m	0.55m
	Total Capacity of AES System pipe in Litres	1696	ltr.		Sand Depth :	0.75m	0.15m
					Area m2	17.0 m^2	7.0 m^2
	USE CUT LENGTHS OF PIPE IN THIS DESIGN? (ENTER Y)	n					
	IF YOU WISH TO USE A TRENCH EXTENSION DESIGN OPTIO	N ENTER "Y	"		Enter Custon	Width in metre	
AES INFI	ILTRATION FOOT PRINT AREA - $L = Q / (DLR x W)$	Length		Width	Minim	ım AES foot prin	t required
	for this Basic Serial design is	12.600m	x	1.90m	= 88	24.0	m2 total
AES pipes are best centered in the trench parallel to the site slope							
Code	AES System Bill of Materials.				Ch	ankar Environme	ntal Use Only
AES-PIPE	AES 3 metre Lengths required	8	lths				
AESC	AES Couplings required	6	ea				
AESO	AES Offset adaptors	4	ea				
AESODV	AES Oxygen demand vent	1	ea				
AES-IPB	AES 100mm Inspection point base	2	ea		SORELL	Sorell Counc	il
TD Kit 4	4 Hole Distribution Box Kit		ca			ment Application:	
TD Kit 7	7 Hole Distribution Box Kit		ea				rive, Carlton - P1.pdf
VS43-4	Sweet Air Filter VS43-4	3983 (4)	ea		Plans F Date R	Reference:P1 eceived:11/10/202	À
AES DESO	Double Offset Adaptors		ca				
	TOTAL SYSTEM SAND REQUIRED (Estimate Only)	17	m3				
Please ema	nil your AES Calculator (EXCEL FORMAT), Site Layout & AES	Design to					
	designreview@enviro-septic.com.au					designreview@envi	iro-septic.com.au

- > The AES Calculator is a design aid to allow checking of the AES components, configuration and is a guide only. Site and soil conditions referencing AS1547 are calculated and designed by a Qualified Wastewater Designer.
- > Chankar Environmental accepts no responsibility for the soil evaluation, loading calculations or DLR entered by the designer for this calculator.
- > AES pipes can be cut to length on site. They are supplied in 3 meter lengths only.
- > AES ONLY supply AES components as detailed in the Bill of Materials.
- > SEPTIC Tank & other components including SAND will need to be sourced from other suppliers. Refer to our WEBSITE www.enviro-septic.com.au OR 07 5474 4055





SITE AND SOIL EVALUATION REPORT

Soil Category:

Modified Emerson Test Required No 1,...2,...3,...4,...5,...6 If Yes, Emerson Class No. Measured or Estimated Soil Permeability (m/d): 1.5m/d Design Loading Rate (DLR) 30 mm/day Geology: Quaternary sediments Slope: 8 degrees Drainage lines / water courses: Nil Vegetation: Grass Site History: (land use) Vacant block Aspect: south Pre-dominant wind direction: Northwest to southwest Site Stability: Will on-site wastewater disposal affect site stability? No Is geological advice required? No Drainage/Groundwater: Not Encountered Depth to seasonal groundwater (m): Not Encountered Are surface or sub-surface drains required upslope of the land application area? No Date of Site Evaluation: 5/3/2024 Weather Conditions: Fine



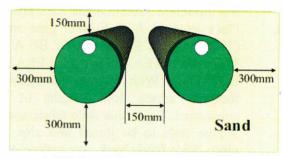
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ENVIRO-SEPTIC™ Advanced Enviro-Septic™ Installation Instructions

1. SET OUT

- Set out should be in accordance with the design approved by Council.
- ii. The length of each run of AES System pipe must be horizontal
- iii. AES calculator footprint dimensions are based upon the DLR of the receiving soil and are the minimum foot print area.
- iv. Any system extension must be to the down slope side unless the infiltration footprint is level.



AES Sand Coverage Minimums

2. EXCAVATION – (track machinery causes less compaction of the soil.)

i. Excavate as required leaving the base of excavation loose to aid infiltration. Strip and separate top soil for covering installation as per AS 1547:2012.

DO NOT damage infiltration area by driving equipment or walking on excavation prior to placement of sand layer. Refer to Appendix L Sec L7 of AS1547: 2012. Construction Techniques. Rip or scarify the infiltration area to a depth of 150 to 200mm minimum parallel to the AES pipe on all systems especially systems in Cat 4,5,6 soil with high clay content. (Refer to the design and report for this onsite installation)

"L7.1 Good construction technique AS 1547:2010

The following excavation techniques shall be observed so as to minimise the risk of damage to the soil:

- (a) Plan to excavate only when the weather is fine:
- (b) Avoid excavation when the soil has a moisture content above the plastic limit. This can be tested by seeing if the soil forms a 'wire' when rolled between the palms;
- (c) During wet seasons or when construction cannot be delayed until the weather becomes fine, smeared soil surfaces may be raked to reinstate a more natural soil surface, taking care to use fine tines and only at the surface;
- (d) When excavating by machine, fit the bucket with 'raker teeth' if possible, and excavate in small 'bites' to minimise compaction; and
- (e) Avoid compaction by keeping people off the finished trench or bed floor.

In particular for trenches and beds:

- (f) If rain is forecast then cover any open trenches, to protect them from rain damage;
- (g) Excavate perpendicular to the line of fall or parallel to the contour of sloping ground; and
- (h) Ensure that the inverts are horizontal.

CL7.1

Damage can be done by:

- (a) Smearing, where the soil surface is smoothed, filling cracks and pores;
- (b) Compacting, where the soil porosity is reduced; and
- (c) Puddling, where washed clay settles on the base of the trench to form a relatively impermeable layer.

In particular, cohesive soils, or soils containing a significant quantity of clay, are susceptible to damage by excavation equipment during construction.

ii. If using a raised bed configuration ensure you have sufficient soil to cover entire mound or bring in enough sand to fill out batters prior to covering with topsoil etc. as per AS 1547:2012.



Sorell Council

Development Application: Development
Application - 28 Riviera Drive, Carlton - P1.pd

Plans Reference:P1

3. SYSTEM SAND - Course washed sand with less than 2mm silt (ASTM C-33)

- i. Place minimum150mm system sand to extension area and minimum 300mm under AES pipe footprint area.
- ii. Place runs of AES System pipe roughly in position (THE FABRIC SEAM MUST BE AT THE TOP AND THE WHITE BIO-ACCELERATOR AT THE BOTTOM.) With 300mm minimum clearance to all footprint edges. Join lengths of AES with AES connectors. To do this slide fabric and fibre back on the 2 pipe ends to be joined and clip AES connector in place. Slide fabric back over connector.
- iii. Place offset adaptors on each run with the 100mm hole at the top.
- iv. Ensure minimum 150mm between AES system pipes.

 This can be done with pegs, short pieces of 150mm pvc or reusable AES Spacer Plates. One side provides the 300m spacing required for minimum system sand. The opposite side must have a minimum of 300mm of system sand beyond the edge of the AES System pipe.
- v. Place system sand around AES pipes ensuring they stay level and in position. Remove and progressively position spacer plates or PVC pipe until all system pipes are surrounded by system sand to the top. Walk sand between rows to aid compaction.
- vi. EXTENSION SAND depth is a minimum of 150mm.

4. CONNECTING ROWS

i. Connect rows with 100mm pipe as required with a maximum 100mm extending into the AES system pipe. (Raised connection – After placing raised connection pipes the top of the PVC pipe must be level with the top of the AES pipe. Lift and pack with sand.) This ensures airflow is not restricted and buffer capacity is maximised.

5. VENTING

- i. Ensure the system has a High Vent and a low vent. As per design. Low vent is a minimum 150mm above ground. Vents can be located any distance from the system provide they have no water traps that can block oxygen flow through the system. The High Vent must be 3 meters higher than the low vent.
- ii. Pressurised or steep gravity systems will require a Velosity Diffuser

6. BACK FILLING

- i. Ensure a minimum of 150mm System sand covers the AES pipes and PVC pipe work.
- ii. Refer to the Onsite design and Council approval and ensure that all diversions drains or site specific requirements are correctly installed.
- iii. Back fill with natural soil and compact. System extensions may require compaction in a couple of layers depending on the depth.
- iv. On mounds and down slopes strip vegetation and place fill evenly and level to all sides to avoid breakout from low points during high seasonal loadings.
- v. Cover excavation area with topsoil creating a finished surface level 50 to 100mm higher than the natural surface level ensuring that water sheds off the land application area and does not pond, compact lightly and seed or grass when completed.







Sorell Council

ate Received:11/10/2024

Development Application: Development pplication - 28 Riviera Drive, Carlton - P1.pdf

CERTIFICATE OF THE RESPONSIBLE DESIGNER

To: Olivia & Dan Maranesi

Section 94 Section 106 Section 129 Section 155

Owner name

	olivia_maranesi@hotmail.com		Address	Form 35				
]]				
						Suburb/postcod	de la	
Designer detail	ls:							
Name:	Peter Hofto					Category:	Building Services Designer Hydraulic - Restricted	
Business name:	Rock Solid Geotechnics P/L					Phone No:	0417960769	
Business address:	163 Orielton Road							
	Orielton			7172		Fax No:		
Licence No:	CC6159I Email a	address: [ре	ter@ro	cksoli	dgeotechnics.c	om.au	
Details of the p	roposed work:							
Owner/Applicant	Olivia & Dan Maranesi					Designer's proj reference No.	ect GEOTECH 24-032	
Address:	28 Riviera Drive, Carlton					Lot No	D:	
							<u> </u>	
Type of work:	Building w	ork 🗌			F	Plumbing work	(X all applicable)	
Description of wor	rk:							
ONSITE WASTEWATER MANAGEMENT SYSTEM Sorell Council Development Application: Development Application: Development Application - 28 Riviera Drive, Carlton - P1.pdf Plans Reference:P1 Date Received:11/10/2024 Description of the Design Work (Scope, limitations or exclusions): (X all applicable certificates)				e-erection vater / sewerage / ormwater / n-site wastewater anagement system / ackflow prevention / other)				
Certificate Type:	Certificate		-	Kolusik		ponsible Pra	Tours :	
	☐ Building design					nitect or Buildi		
	☐ Structural design					Engineer or Civil Designer		
	☐ Fire Safety design				Fire	Fire Engineer		
	☐ Civil design				Civil	ivil Engineer or Civil Designer		
	X Hydraulic design				Buile	ding Services	Designer	
	☐ Fire service design				Build	ding Services	Designer	
	☐ Electrical design				Build	ding Services	Designer	
	☐ Mechanical design				Build	ding Service D	Designer	
	Plumbing design				Plun Des	nber-Certifier; igner or Engir	Architect, Building neer	
	☐ Other (specify)							
Deemed-to-Satisfy:	X	Perfor	ma	nce So	olutio	n: (X the ap	ppropriate box)	
Other details:								

Design documents provided: The following documents are provided with this Certificate -Document description: Drawing numbers: Prepared by: ROCK SOLID GEOTECHNICS Date: 8/3/2024 Date: Schedules: Prepared by: Specifications: Date: 8/3/2024 Prepared by: ROCK SOLID GEOTECHNICS Computations: Prepared by: ROCK SOLID GEOTECHNICS Date: 8/3/2024 Prepared by: Date: Performance solution proposals: Prepared by: Date: Test reports: Standards, codes or guidelines relied on in design process: AS 1547:2021 On-site domestic wastewater management **Sorell Council** Director's Guidelines for Onsite Wastewater Management velopment Application: Development cation - 28 Riviera Drive, Carlton - P1.pdf Plans Reference:P Date Received:11/10/2024 Any other relevant documentation: Advanced Enviro Septic Design & Installation Manual. Advanced Enviro Septic Design Installation Instructions & Home-Owner's Manual. All by Chandlers Environmental Pty Ltd Site & Soil Evaluation and design report, 28 Riviera Drive, Carlton, dated 8/3/2024 Form 55 by Rock Solid Geotechnics P/L, dated 8/3/2024, certifying Site & Soil Evaluation Report

Attribution as designer:

Peter Hofto – ROCK SOLID GEOTECHNICS P/L am responsible for the design of that part of the work as described in this certificate;

The documentation relating to the design includes sufficient information for the assessment of the work in accordance with the *Building Act 2016* and sufficient detail for the builder or plumber to carry out the work in accordance with the documents and the Act;

This certificate confirms compliance and is evidence of suitability of this design with the requirements of the National Construction Code.

	Name: (print)	Signed	Date
Designer:	Peter Hofto	9/101	8/3/2024
Licence No:	CC6159I		

Assessment of Certifiable Works: (TasWater) Note: single residential dwellings and outbuildings on a lot with an existing sewer connection are not considered to increase demand and are not certifiable. If you cannot check ALL of these boxes, LEAVE THIS SECTION BLANK. TasWater must then be contacted to determine if the proposed works are Certifiable Works. I confirm that the proposed works are not Certifiable Works, in accordance with the Guidelines for TasWater CCW Assessments, by virtue that all of the following are satisfied: The works will not increase the demand for water supplied by TasWater | x | The works will not increase or decrease the amount of sewage or toxins that is to be removed by, or discharged into, TasWater's sewerage infrastructure The works will not require a new connection, or a modification to an existing connection, to be made to TasWater's infrastructure x The works will not damage or interfere with TasWater's works x The works will not adversely affect TasWater's operations The work are not within 2m of TasWater's infrastructure and are outside any TasWater easement I have checked the LISTMap to confirm the location of TasWater infrastructure If the property is connected to TasWater's water system, a water meter is in place, or has been applied for to TasWater. Certification: IPeter Hofto – ROCK SOLID GEOTECHNICS P/L..... being responsible for the proposed work, am satisfied that the works described above are not Certifiable Works, as defined within the Water and Sewerage Industry Act 2008, that I have answered the above questions with all due diligence and have read and understood the Guidelines for TasWater CCW Assessments.

Note: the Guidelines for TasWater Certification of Certifiable Works Assessments are available at: www.taswater.com.au

	Name: (print)	Signed		Date
Designer:	Peter Hofto	PHO		8/3/2024
			-	

CERTIFICATE OF QUALIFIED PERSON – ASSESSABLE ITEM

Section 321

To:	Olivia & Dan Maranesi			Owner /Agent			
	olivia_maranesi@hotmail.com			Address	Form	55	
				Suburb/postcod⊛			
Qualified person	on details:						
Qualified person:	Peter Hofto - Rock Solid Geotechnics P	/L					
Address:	163 Orielton Road			Phone No:		0417960769	
	Orielton	22 (2004 - 2004 - 2004 - 2004 - 2004 - 2004 - 2004 - 2004 - 2004 - 2004 - 2004 - 2004 - 2004 - 2004 - 2004 - 2					
Licence No:	CC6159I Email ad	ddress:	peter@	procksolidgeotech	nics.com	.au	
Qualifications and Insurance details: Speciality area of expertise:	BSc (Hons) – Geology / Geophysics PI Insurance – Lloyds Underwriting Policy Number ENG 22000391 Site & Soil Evaluation & Land Applicatio System Design	Directo by Qual Items Number ENG 22000391 Soil Evaluation & Land Application Director Direc			escription from Column 4 of the rector's Determination - Certificates Qualified Persons for Assessable		
Details of work							
Address:	28 Riviera Drive, Carlton] ι	ot No:	,	
				Certificate of ti	tle No:	10	
The assessable item related to this certificate:	Onsite wastewater management – site & soil onsite wastewater management capacity, including characterisation of wastewater and predicted loadings, Selection of onsite wastewater treatment systems Selection of land application area, Determination of design loading rate.	uding; hydrau		(description of the certified) Assessable item in a material; a design a form of cons a document testing of a consystem or plun an inspection, performed	ncludes – etruction mponent, nbing syst	building tem	
Certificate deta	ils:						
	On-site wastewater management – Site & Evaluation		Schedule Determina	on from Column 1 of 1 of the Director's ation - Certificates by Persons for Assessa	,		
/	elation to the above assessable items work, plumbing work or plumbing insta				k one)		
OR	5 ·· 2·· promoting moto		or define	ON WOLK			

a building, temporary structure or plumbing installation

Sorell Council

Plans Reference:P1 Date Received:11/10/2024

Development Application: Development Application - 28 Riviera Drive, Carlton - P1.pdf

Documents:	AS 1547:2021 On-site domestic wastewater management
Relevant calculations:	
References:	AS/NZS 1547.2012 - Onsite domestic wastewater management Director's Guidelines for Onsite Wastewater Management – CBOS - 2017
	Substance of Certificate: (what it is that is being certified)
dated 23/2/2024	& Design Report - 368 Primrose Sands Road, Primrose Sands by Rock Solid Geotechnics P/L
	Scope and/or Limitations
Exclusions: Design o	f AES Bed
I certify the matter Qualified person:	s described in this certificate. Signed: GEOTECH 24-032
	Sorell Council Development Application: Development Application - 28 Riviera Drive, Carlton - P1.pdf

CERTIFICATE OF THE RESPONSIBLE DESIGNER

Section 94 Section 106 Section 129 Section 155

To:	Olivia & Dan Maranesi				Owner name	2 E	
	olivia_maranesi@hotmail.co	<u>m</u>			Address	Form 35	
					Suburb/postcoo	le .	
Designer detai	ls:						
Name:	Stephen John Dennis				Category:	Building Services Designer Hydraulic - Restricted	
Business name:					Phone No:	0417960769	
Business address:	4/100 Rene Street						
	Noosaville		45	66	Fax No:		
Licence No:	373083211 Ema	ail address:	steve@	@enviro-	septic.com.au		
Details of the p	proposed work:						
Owner/Applicant	Olivia & Dan Maranesi				Designer's proje reference No.	GEOTECH 24-032	
Address:	28 Riviera Drive, Carlton				Lot No):	
]		
Type of work:	Building	work		,	Plumbing work	X (X all applicable)	
Description of wor	rk:					ew building / alteration /	
ONSITE WASTEWAT	TER MANAGEMENT SYSTEM			> 4.5	ac re w ste or ma	Idition / repair / removal / -erection vater / sewerage / ormwater / n-site wastewater anagement system / nckflow prevention / other)	
Description of the	Design Work (Scope, lim	itations or	exclu	sions)	: (X all applicable	certificates)	
Certificate Type:	Certificate			7 7 7 8 8	sponsible Pra		
	X Hydraulic design			Bui	uilding Services Designer		
	Other (specify)						
Deemed-to-Satisfy:		Perfor	mance	Solution	on: X(X the ap)	propriate box)	
Other details:		'		a-to-base fac-		· · · · · · · · · · · · · · · · · · ·	
Advanced Enviro Sept	consistent with NCC Vol 3 wit tic unit producing secondary to wastewater management syste	eated effluer	: nt cons	istent w	ith definition pro	vided by Director's	
				Г	SORELL		
					Sorell Coun Development Application		
				/	Application - 28 Riviera		
					Date Received:11/10/20)24	

Design documents provided: The following documents are provided with this Certificate -Document description: Drawing numbers: Prepared by: ROCK SOLID GEOTECHNICS Date: 8/3/2024 Date: Prepared by: Schedules: Specifications: Prepared by: ROCK SOLID GEOTECHNICS Date: 8/3/2024 Computations: Date: 8/3/2024 Prepared by: ROCK SOLID GEOTECHNICS Prepared by: Stephen Dennis Performance solution proposals: Date: 8/3/2024 Date: Prepared by: Test reports: Standards, codes or guidelines relied on in design process: AS 1547:2021 On-site domestic wastewater management **Sorell Council** Director's Guidelines for Onsite Wastewater Management velopment Application: Development olication - 28 Riviera Drive, Carlton - P1.pdf Plans Reference:P1 Date Received: 11/10/2024 Any other relevant documentation: Advanced Enviro Septic Design & Installation Manual. Advanced Enviro Septic Design Installation Instructions & Home-Owner's Manual. All by Chandlers Environmental Pty Ltd

Site & Soil Evaluation and design report by Rock Solid Geotechnics P/L for 28 Riviera Drive, Carlton, dated 8/3/2024

Form 55 by Rock Solid Geotechnics P/L, dated 8/3/2024, certifying Site & Soil Evaluation Report

Attribution as designer:

Peter Hofto – ROCK SOLID GEOTECHNICS P/L am responsible for the design of that part of the work as described in this certificate;

The documentation relating to the design includes sufficient information for the assessment of the work in accordance with the *Building Act 2016* and sufficient detail for the builder or plumber to carry out the work in accordance with the documents and the Act;

This certificate confirms compliance and is evidence of suitability of this design with the requirements of the National Construction Code.

	Name: (print)	Signed	Date
Designer:	Stephen Dennis		8/3/2024
Licence No:	373083211		

Assessment of Certifiable Works: (TasWater)					
Note: single residential dwellings and outbuildings on a lot with an existing sewer connection are not considered to increase demand and are not certifiable.					
If you cannot check ALL of these boxes, LEAVE THIS SECTION BLANK.					
TasWater must then be contacted to determine if the proposed works are Certifiable Works.					
I confirm that the proposed works are not Certifiable Works, in accordance with the Guidelines for TasWater CCW Assessments, by virtue that all of the following are satisfied:					
x The works will not increase the demand for water supplied by TasWater					
The works will not increase or decrease the amount of sewage or toxins that is to be removed by, or discharged into, TasWater's sewerage infrastructure					
The works will not require a new connection, or a modification to an existing connection, to be made to TasWater's infrastructure					
The works will not damage or interfere with TasWater's works					
The works will not adversely affect TasWater's operations					
The works are not within 2m of TasWater's infrastructure and are outside any TasWater easement					
x I have checked the LISTMap to confirm the location of TasWater infrastructure					
x If the property is connected to TasWater's water system, a water meter is in place, or has been applied for to TasWater.					
Certification:					

proposed work, a the <i>Water and S</i> diligence and ha	Dennisam satisfied that the works describe ewerage Industry Act 2008, that I have read and understood the Guidelielines for TasWater Certification ter.com.au	ed above are not Certifiable Works, ave answered the above questions ines for TasWater CCW Assessmer	as defined within with all due hts.
	Name: (print)	Signed	Date
Designer:	Stephen Dennis		8/3/2024



CERTIFICATE OF QUALIFIED PERSON – ASSESSABLE ITEM

Section 321

То	: Olivia & Dan Maranesi	Olivia & Dan Maranesi		
	olivia_maranesi@hotmail.com	olivia_maranesi@hotmail.com		
			Suburb/postcode	
Qualified pers	on details:			
Qualified person:	Stephen John Dennis		1	
Address:	PO Box 1556		Phone No:	0455 826 203
	Noosaville	4566	Fax No:	
Licence No:	373083211 Email addr	ess: stev	edennis913@gmail.	.com
Qualifications and Insurance details:	BEng(Hons), GradDipMgt	Directo	iption from Column : or's Determination - alified Persons for A	Certificates
Speciality area of expertise:	Professional Engineering Wastewater Design NER, NPER, CPEng, APEC, IntPE(Aust), RPEQ20663	(descr Direct	ription from Column or's Determination - alified Persons for A	Certificates
Details of wor	k:			
Address:	28 Riviera Drive, Carlton] .	Lot No:
			Certificate of ti	itle No:
The assessable item related to this certificate:	Design of an onsite wastewater management system.		(description of the assessable item bein certified) Assessable item includes — - a material; - a design - a form of construction - a document - testing of a component, building system or plumbing system - an inspection, or assessment, performed	
Certificate deta	ails:			
Certificate type:	Design of an onsite wastewater management system.	Schedule Determina	on from Column 1 of 1 of the Director's ation - Certificates by Persons for Assessa	у

This certificate is in relation to the above assessable items, at any stage, as part of – (tick one)

building work, plumbing work or plumbing installation or demolition work

OR

a building, temporary structure or plumbing installation



Development Application: Development Application - 28 Riviera Drive, Carlton - P1.pdf

In issuing this certification	ate the following matters are relevant –
Documents:	Geotech 24-032 Rock Solid Geotechnics P/L
Relevant calculations:	
References:	NCC Vol 3. Refer to AES Tasmania NCC Performance Solution V4. AS/NZS 1547.2012 - Onsite domestic wastewater management Director's Guidelines for Onsite Wastewater Management 2017 Advanced Enviro Septic Design & Installation Manual, Advanced Enviro-Septic Installation Instructions and, Home Owner's Manual; all by Chankar Environmental Pty Ltd, 62 Rene Street, Noosaville QLD 4566
	Substance of Certificate: (what it is that is being certified)
	performance solution for design of Advanced Enviro-Septic System at 28 Riviera Drive, Carlton. ance with NCC Vol 3 TAS Section H is provided in the appended document headed "AES Tasmanian Solution")
2	Scope and/or Limitations
Exclusions: All works	other than the above.
I certify the matters	s described in this certificate.
Qualified person:	Signed: Certificate No: Date: GEOTECH 8/3/2024 24-032 24-032



Development Application: Development Application - 28 Riviera Drive, Carlton - P1.pdf

Olivia & Dan Maranesi olivia_maranesi@hotmail.com ROCK SOLID GEOTECHNICS PTY LTD
Peter Hofto
163 Orielton Rd

Orielton

TAS 7172

0417960769

peter@rocksolidgeotechnics.com.au

8/3/2024

Loading Certificate for Onsite Wastewater System

28 Riviera Drive, Carlton

1 System Capacity:

(medium/long term)
 4-bedroom residence, 6 persons, 720 litres/day

2 Design Criteria Summary:

Primary Treated Effluent 3250 litre Dual-purpose septic tank.

Soil Category
 Class 1 SAND

Land Application System
 12.6m long x 1.90m wide AES Bed

3 Reserve Area:

Reserve LAA available if required.

- 4 Variation from design flows etc:
 - The system should successfully assimilate additional peak loadings which may result from occasional social gatherings provided that this does not exceed use by more than 10 persons in a 24-hour period or more than 2 temporary resident visitors (ie. up to 8 persons total) for a period not exceeding 4 days. Visitors should be advised of the requirement to minimise time spent in showers, not running taps whilst cleaning teeth, and other common sense water conservation measures.
- 5 Consequences of overloading the system:
 - Long term use by more than 6 residents or equivalent may result in overloading of the system, surfacing of effluent, public and environmental health nuisances, pollution of surface water etc.
- 6 Consequences of under-loading the system:
 - Nil.
- 7 Consequences of lack of operation, maintenance and monitoring attention:
 - The septic tank should be pumped at least every 3 years.



Rock Solid Geotechnics Pty Ltd



Development Application: Development Application - 28 Riviera Drive, Carlton - P1.pdf 12

PLANNING

28 RIVIERA DRIVE **CARLTON, TASMANIA, 7173 OLIVIA MARANESI & DANIEL WOODWARD**

REV P1 - ISSUE FOR PLANNING APPROVAL 26.09.2024

GENERAL INFORMATION:

DRAWING REGISTER:

DESIGNER: DAVID LYNE CC7063	B00	COVER SHEET	N/A
CLIENT: OLIVIA & DAN	B01	NOTES SHEET 1	N/A
JOB NUMBER: 1566/24	B02	NOTES SHEET 2	N/A
CLASSIFICATION: 1a	B03	SITE PLAN	1:200
TITLE REFERENCE: 161552/3	B04	FLOOR PLAN	1:100
DESIGN WIND SPEED: 'N3'	B05	ELEVATIONS	1:100
SOIL CLASSIFICATION: 'S'	B07	SHED PLANS	1:100

CLIMATE ZONE: 7

BAL: N/A

LOT SIZE: 2009m²

PROPOSED NEW FLOOR AREA: 111m² PROPOSED DECK AREA: 23.6m² PROPOSED SHED AREA: 26m²

GENERAL NOTES:

- CHECK ALL DIMENSIONS, BOUNDARIES, EASEMENTS AND SERVICE LOCATIONS ON SITE.
- ALL WORK SHALL COMPLY WITH THE TASMANIAN BUILDING REGULATIONS 2016, AND RELEVANT CURRENT AUSTRALIAN STANDARDS.
- CHECK CAREFULLY ALL ASPECTS OF THESE DOCUMENTS BEFORE COMMENCING WORK.
- ANY ERRORS OR ANOMALIES TO BE REPORTED TO THE DRAWER BEFORE WORK IS CONTINUED.
- CONFIRM ALL SIZES AND HEIGHTS ON SITE. DO NOT SCALE OFF PLAN.
- ALL FRAMING TO COMPLY WITH AS 1684 RESIDENTIAL TIMBER-FRAMED CONSTRUCTION, NOTE: ALL TIMBER SIZES SPECIFIED ARE MINIMUM REQUIREMENT ONLY.
- SUBSTITUTES MAY BE USED AS LONG AS VERIFICATION OF EQUAL PERFORMANCE IS OBTAINED.
- ALL CONSTRUCTION IS TO COMPLY WITH THE BUILDING CODE OF AUSTRALIA AND ALL RELEVANT AUSTRALIAN STANDARDS.
- WIND LOADS DETERMINED IN ACCORDANCE WITH AS 4055 WIND LOADS FOR HOUSING.
- THESE DOCUMENTS TO BE USED WITH THE DESIGNERS SPECIFICATIONS, SOIL TESTS AND ALL DOCUMENTATION PREPARED BY AN ENGINEER.
- THESE DOCUMENTS ARE INTENDED FOR COUNCIL APPLICATIONS AND NORMAL CONSTRUCTION.
- THIS DESIGN IS COVERED UNDER COPYRIGHT AND ANY CHANGES MUST BE CONFIRMED WITH THE DESIGNER, THE DESIGNER RETAINS ALL INTELLECTUAL PROPERTY.



PLEASE READ CAREFULLY

THIS PLAN CERTIFIED CORRECT IS THE ONE REFERRED TO IN THE BUILDING CONTRACT AND I UNDERSTAND CHANGES HEREAFTER MAY NOT BE POSSIBLE.

FINAL PLAN: ANY REQUESTED VARIATIONS TO YOUR HOUSE PLAN WILL INCUR AN AMENDMENT / ADMINISTRATION MINIMUM FEE

> P1 26.09.2024 PRELIMINARY ISSUE REV DATE REMARK

LYNE DESIGN

BUILDING DESIGN/ DRAFTING - BUSHFIRE MANAGEMENT DAVID LYNE 11 GRANVILLE AVENUE ACCREDITED DESIGNER: CC7063 GEILSTON BAY, TASMANIA 7015 MOBILE: 0421 852 987 daye lyne@hotmail.com

28 RIVIERA DRIVE CARLTON, TAS 7173

COVER SHEET

Accepted (Client 1)	OLIVIA MARANESI	Date
Accepted (Client 2)	DANIEL WOODWARD	Date
Approved (Builder)	NOT APPLICABLE	Date

This document must be signed

SCALES @ A3	DESIGNED BY	DRAWN BY
N/A	D.LYNE	D.LYNE
	PLOT DATE	09/10/2024

DO NOT SCALE. Use only figured dimensions. Locations of structure, fittings, services etc on this drawing are indicative only. CONTRACTOR to check all other project drawings for co-ordination between structure, fabric, fixtures, fittings, services etc. CONTRACTOR to site check all dimensions and exact locations of all items. no responsibility shall be taken for dimensional information scaled or digitally derived from this document.

PLOT DETAILS RIVIERA.DWG

PROJECT NO. 1566/24

B00

REVISION P1 Copyright @ All rights reserved. This drawing and its intellectual content remains

the intellectual property of david lyne.

The recipient client is licensed to use this drawing for its commissioned purpos subject to authorisation per note above. Unicensed use is prohibited. Unicense parties may not copy, reproduce or retransmit or amend this document or any part or this document without consent. Amendment of this document is prohibited.

GENERAL:

- THESE NOTES MUST BE READ AND UNDERSTOOD BY ALL INVOLVED IN THE PROJECT. (THIS INCLUDES BUT IS NOT LIMITED TO THE OWNER. BUILDER, SUB-CONTRACTORS, CONSULTANTS, OPERATORS, RENOVATORS, MAINTAINERS AND DEMOLISHERS.
- THESE DRAWINGS ARE TO BE READ IN CONJUNCTION WITH THE STRUCTURAL DESIGN DRAWINGS, ANY OTHER DRAWINGS BY OTHERS. SPECIFICATIONS AND OR REPORTS, AND WITH SUCH OTHER WRITTEN INSTRUCTION THAT MAY BE ISSUED.
- ALL CONSTRUCTION WORK TO COMPLY WITH THE CURRENT BUILDING CODES AND REGULATIONS OF THE RELEVANT AUSTRALIAN STATE OR
- DIMENSIONS ARE EXPRESSED IN MILLIMETERS, UNO. DIMENSIONS SHALL NOT BE OBTAINED BY SCALING THESE DRAWINGS. SETTING OUT DIMENSIONS SHALL BE VERIFIED BY LICENCED BUILDER/ SURVEYOR BEFORE COMMENCING WORK. ANY DISCREPANCIES BETWEEN THESE DRAWINGS AND THE OTHER DRAWINGS IS TO BE ADVISED BEFORE PROCEEDING WITH ANY WORK.
- THE PLANS AND SPECIFICATIONS ARE SUBJECT TO APPROVAL BY THE LOCAL BUILDING AUTHORITY PRIOR TO CONSTRUCTION.
- THE BUILDING CONTRACTOR SHALL OBTAIN ALL NECESSARY PERMITS, APPROVALS, AND INSPECTIONS AS REQUIRED BY LAW.
- ALL STRUCTURAL ELEMENTS, INCLUDING FOOTINGS, FOUNDATIONS, WALLS, AND ROOF STRUCTURES, MUST BE DESIGNED BY A QUALIFIED STRUCTURAL ENGINEER AND COMPLY WITH THE APPROPRIATE AUSTRALIAN STANDARDS. REFER TO STRUCTURAL DRAWINGS BEFORE
- THE MATERIALS AND FINISHES SPECIFIED MUST MEET THE REQUIRED AUSTRALIAN STANDARDS AND REGULATIONS FOR FIRE SAFETY. ACOUSTIC PERFORMANCE, ENERGY EFFICIENCY, AND DURABILITY.
- THE BUILDER SHALL CARRY OUT REGULAR QUALITY INSPECTIONS DURING CONSTRUCTION TO ENSURE COMPLIANCE WITH THE PLANS AND SPECIFICATIONS.
- THE HOUSE PLANS ARE COPYRIGHT PROTECTED, AND UNAUTHORIZED REPRODUCTION, OR USE WITHOUT THE WRITTEN CONSENT OF THE DESIGNER IS STRICTLY PROHIBITED.
- 11. THE FINAL COMPLETION OF THE PROJECT SHALL INCLUDE OBTAINING THE NECESSARY OCCUPANCY CERTIFICATE FROM THE LOCAL BUILDING AUTHORITY

HEALTH AND SAFTEY:

- THERE ARE A NUMBER OF HAZARDS AND HENCE RISKS WHICH ARE NOT UNIQUE TO THIS PROJECT WHICH NEED TO BE MANAGED DURING THESE PHASES. A REMINDER TO CONSTRUCTORS, OPERATORS, MAINTAINERS AND DEMOLISHERS OF THEIR RESPONSIBILITIES UNDER WORK HEALTH & SAFETY ACTS AND REGULATIONS.
- 2. FOR ALTERATIONS TO OR DEMOLITION OF A BUILDING CONSTRUCTED
 - 1990 IT MAY CONTAIN ASBESTOS 1986 - IT IS LIKELY TO CONTAIN ASBESTOS EITHER IN CLADDING MATERIAL, OR IN FIRE-RETARDANT INSULATION MATERIAL IN EITHER CASE, THE BUILDER SHOULD CHECK, AND IF NECESSARY TAKE APPROPRIATE ACTION BEFORE DEMOLISHING, CUTTING, SANDING, DRILLING OR OTHERWISE DISTURBING THE EXISTING STRUCTURE.
- MANY OTHER MATERIALS USED IN CONSTRUCTION OF THIS BUILDING SUCH AS (BUT NOT LIMITED TO) POWDERED MATERIALS, TREATED TIMBER, VOLATILE ORGANIC COMPOUNDS, SYNTHETIC MINERAL FIBER CAN CAUSE HARM IF INHALED ALL PERSONS WORKING ON OR IN THE BUILDING DURING DEMOLITION AND OR CONSTRUCTION SHOULD BE WEARING PERSONAL PROTECTION EQUIPMENT INCLUDING PROTECTION AGAINST INHALATION OF HARMFUL MATERIAL.

SITE PREPARATION AND EXCAVATION:

- IN ACCORDANCE WITH PART H1D3 OF CURRENT N.C.C., AND TO LOCAL COUNCIL REQUIREMENTS.
- 2. THE SITE MUST BE PROPERLY PREPARED AND LEVELED BEFORE CONSTRUCTION BEGINS. ANY NECESSARY EARTHWORKS AND SITE PREPARATION ARE THE RESPONSIBILITY OF THE CONTRACTOR.
- SURFACE DRAINS SHALL BE DESIGNED AND CONSTRUCTED TO AVOID WATER PONDING AGAINST OR NEAR THE FOOTINGS. THE GROUND IN THE IMMEDIATE VICINITY OF THE PERIMETER FOOTINGS, INCLUDING THE GROUND UPHILL FROM A SLAB ON CUT-AND FILL SITES. SHALL BE GRADED TO FALL 50mm MINIMUM OVER A DISTANCE OF 1m AND SHAPED TO PREVENT PONDING OF WATER. WHERE FILL IS PLACED ADJACENT TO THE BUILDING, THE FILL SHALL BE COMPACTED AND GRADED TO ENSURE DRAINAGE OF WATER AWAY FROM THE BUILDING.
- 4. INTERNAL FINISHED FLOOR LEVEL (FFL) TO BE MIN. 150mm ABOVE FINISHED EXTERNAL GROUND AREAS (FLOWER BEDS OR GRASSED AREAS) AND MIN. 50mm ABOVE FINISHED EXTERNAL SEALED SURFACES (PAVED AREAS). PROVIDE 50mm MIN. FALL FOR THE FIRST METER AWAY FROM BUILDING TOWARDS LOWER GROUND OR ALTERNATIVELY SUFFICIENT DRAINAGE PROVISIONS (AG DRAINS, SUMPS OR SIMILAR).

FOOTINGS & SLABS:

- CONCRETE FOOTINGS AND SLABS ARE TO BE IN ACCORDANCE WITH PART H1D4 OF CURRENT N.C.C. AND AS 2870.1 AND ENGINEERS DOCUMENTATION/ SPECIFICATIONS.
- PROVIDE WALL CAVITY DRAINAGE WITH WEEPHOLES AT 960 MAX CENTERS ALONG LINE ABOVE FINISHED GROUND LEVEL. (SLAB AREA).

PLUMBING (SEWER & STORMWATER):

- 1. ALL PLUMBING WORK SHALL COMPLY WITH THE RELEVANT PLUMBING STANDARDS AND BE CARRIED OUT BY A LICENSED PLUMBER.
- ADEQUATE PROVISIONS FOR STORMWATER DRAINAGE MUST BE MADE TO PREVENT WATERLOGGING AND FLOODING ON THE PROPERTY. THE DRAINAGE SYSTEM DESIGN SHOULD COMPLY WITH THE LOCAL COUNCIL'S GUIDELINES.

GLAZING:

- ALL WINDOWS AND GLAZING MUST COMPLY WITH AS2047 AND AS1288 AND PART H1D1 OF CURRENT N.C.C. MANUFACTURER TO PROVIDE CERTIFICATION OF COMPLIANCE.
- ALL WINDOW MEASUREMENTS SHOWN ARE NOMINAL ONLY AND ARE TO BE VERIFIED ON SITE, PRIOR TO ORDERING.

LIGHT:

- 1. NATURAL LIGHT TO BE IN ACCORDANCE WITH PART H4P4 OF CURRENT
- 2. NATURAL LIGHT MUST BE PROVIDED TO ALL HABITABLE ROOMS
- REQUIRED WINDOWS MUST HAVE A LIGHT TRANSMITTING AREA OF AT LEAST 10% OF THE FLOOR AREA.
- SANITARY COMPARTMENTS, BATHROOM, LAUNDRIES AND THE LIKE NOT PROVIDED WITH NATURAL LIGHT MUST BE PROVIDED WITH ARTIFICIAL LIGHT AT A RATE OF NOT LESS THAN 1 LIGHT FITTING PER 16m2 OF FLOOR AREA AND IN ACCORDANCE WITH AS/NZS 1680.0

HEATING APPLIANCES:

- IN ACCORDANCE WITH PART H7D5 OF CURRENT N.C.C AND AS/NZS 2918 FOR DOMESTIC SOLID-FUEL BURNING APPLIANCES.
- 2. ALL HEATING APPLIANCES TO BE INSTALLED IN ACCORDANCE WITH MANUFACTURERS SPECIFICATIONS.

BUILDING SEALING:

- IN ACCORDANCE WITH CURRENT N.C.C. CHIMNEYS AND FLUE MUST BE FITTED WITH A DAMPER/FLAP THAT CAN BE CLOSED.
- ROOFS, WALLS & FLOORS SEALED BY CAULKING, SKIRTING, ARCHITRAVES OR THE LIKE.
- EXTERNAL WINDOWS AND DOORS TO BE FITTED WITH COMPRESSIBLE STRIP, FOAM, RUBBER OR FIBROUS SEAL TO ALL EXTERNAL WINDOW SASHES AND EXTERNAL DOORS.
- 4. EXTERNAL FANS TO BE SELF CLOSING DAMPER OR FILTER TO BE
- 5. ALL DOWNLIGHTS TO BE SEALED.

- SMOKE ALARMS TO BE INSTALLED IN ACCORDANCE WITH PART H3D6 OF CURRENT N.C.C. AND AS3786 REQUIREMENTS.
- SMOKE ALARMS TO BE INSTALLED IN A CLASS 1a BUILDING ON OR NEAR THE CEILING IN: - ANY STOREY CONTAINING BEDROOMS (i) BETWEEN EACH PART OF THE DWELLING CONTAINING BEDROOMS AND THE REMAINDER OF THE DWELLING; AND (ii) WHRE BEDROOMS ARE SERVED BY A HALLWAY, IN THAT HALLWAY.
- ALL SMOKE ALARMS MUST BE CONNECTED TO THE CONSUMER MAINS POWER WHERE CONSUMER POWER IS SUPPLIED TO THE BUILDING
- SMOKE ALARMS TO BE INTERCONNECTED WHERE THERE IS MORE THAN 1 ALARM
- SMOKE ALARMS TO HAVE BATTERY BACK UP IN CASE OF POWER OUTAGE. RECOMMENDED BATTERIES MUST BE CAPABLE OF SERVICING THE SMOKE ALARM WITH A FAULT FOR 1 YEAR.

ROOF CLADDING, GUTTERING AND DOWNPIPES:

- IN ACCORDANCE WITH H2D6 OF CURRENT N.C.C., AS/NZS 3500.3 AND AS/NZS 3500.5. SECTION 5 INSTALLATION TO BE IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS AND RECOMMENDATIONS.
- WHEREVER FULL SHEETS CAN'T BE UTILIZED PROVIDE THE DISTANCE FOR END LAPPING WHERE SHEETS MEET: - SLOPES 5-15 = A LAP OF 200mm - SLOPES 15 AND GREATER = A LAP OF
- GUTTERS MUST BE INSTALLED WITH A FALL OF NOT LESS THAN -1:500 FOR EAVES GUTTERS AND 1:100 FOR BOX GUTTERS.
- DOWNPIPES MUST:
- NOT SERVE MORE THAN 12m OF GUTTER LENGTH FOR EACH DOWNPIPE.
- BE LOCATED AS CLOSE AS POSSIBLE TO VALLEY GUTTERS AND IF THE DOWNPIPE IS MORE THAN 1.2m FROM A VALLEY, PROVISION FOR OVERFLOW MUST BE MADE TO THE GUTTER.

WET AREAS & EXTERNAL WATERPROOFING:

- 1. IN ACCORDANCE WITH PART H4D2 OF CURRENT N.C.C. AND AS/NZS 3740.
- BUILDING ELEMENTS IN WET AREAS MUST BE WATERPROOF OR WATER RESISTANT IN ACCORDANCE WITH TABLE UNDER H4D4.
- WATERPROOFING MEMBRANES FOR EXTERNAL ABOVE GROUND USE MUST COMPLY WITH AS4654 PARTS 1 AND 2.



lans Reference:P1 ate Received:11/10/2024

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FINAL PLAN: ANY REQUESTED VARIATIONS TO YOUR HOUSE PLAN WILL INCUR AN AMENDMENT / ADMINISTRATION MINIMUM FEE

> P1 26.09.2024 PRELIMINARY ISSUE REV DATE REMARK

LYNE DESIGN

BUILDING DESIGN/ DRAFTING - BUSHFIRE MANAGEMENT ACCREDITED DESIGNER: CC7063 11 GRANVILLE AVENUE

GEILSTON BAY, TASMANIA 7015 MOBILE: 0421 852 987 dave_lyne@hotmail.com

PROJECT.

28 RIVIERA DRIVE CARLTON, TAS 7173

NOTES SHEET

Date
Date
Date

SCALES @ A3 DESIGNED BY DRAWN BY D. LYNE D. LYNE PLOT DATE | 09/10/2024

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PLOT DETAILS RIVIERA.DWG

PROJECT NO. 1566/24

DWG NO

REVISION

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STAIR CONSTRUCTION:

- 1. IN ACCORDANCE WITH PART H5D1 OF CURRENT N.C.C.
- 2. STAIRS MUST BE CONSTRUCTED IN ACCORDANCE WITH THE FOLLOW: (a) EACH FLIGHT MUST HAVE NOT MORE THAN 18 NOR LESS THAN 2 RISERS. (b) THE NOMINAL DIMENSION OF GOINGS AND RISERS OF A STAIR MUST BE CONSTANT THROUGHOUT EACH STAIR (c) TREADS MUST BE SOLID CONSTRUCTION IF THE STAIRWAY IS MORE THAN 10m HIGH OR 3 STOREYS. (d) A FLIGHT OF STAIRS MUST NOT HAVE MORE THAN 3 WINDERS (e) THE RISERS MUST NOT ALLOW A 125mm SPHERE TO PASS THROUGH BETWEEN THE TREADS (f) TREADS & NOSINGS MUST HAVE-(i) A SURFACE WITH A SLIP-RESISTANCE CLASSIFICATION NOT LESS THAN THAT LISTED IN TABLE UNDER H5D2 WHEN TESTED IN ACCORDANCE WITH AS4586

BARRIERS AND HANDRAILS:

- 1. IN ACCORDANCE WITH PART H5P1 OF CURRENT N.C.C.
- (a) A CONTINUOUS BARRIER MUST BE PROVIDED ALONG THE SIDE OF -(i) ANY ROOF TO WHICH GENERAL ACCESS IS PROVIDED; AND (ii) ANY STAIRWAY OR RAMP; AND (iii) A FLOOR, CORRIDOR, HALLWAY, DECK, VERANDAH, MEZZANINE OR THE LIKE; AND (iv) ANY DELINEATED PATH OF ACCESS TO A BUILDING (b) THE REQUIREMENTS OF (a) DO NOT APPLY TO -(i) AREAS REFERRED TO IN H5D2; OR (ii) A RETAINING WALL, UNLESS THE RETAINING WALL FORMS PART OF, OR IS DIRECTLY ASSOCIATED WITH A DELINEATED PATH OF ACCESS TO A BUILDING FROM THE ROAD, OR A DELINEATED PATH OF ACCESS BETWEEN BUILDINGS: OR (iii) A BARRIER PROVIDED TO AN OPENABLE WINDOW COVERED BY H5D2
- (a) THE HEIGHT OF A BARRIER REQUIRED BY H5D3 MUST BE IN ACCORDANCE WITH THE FOLLOWING: (i) THE HEIGHT MUST NOT BE LESS THAN 865mm ABOVE THE NOSINGS OF THE STAIR TREADS OR THE FLOOR OF A RAMP. (ii) THE HEIGHT MUST NOT BE LESS THAN -(A) 1m ABOVE THE FLOOR OF ANY ACCESS PATH, BALCONY, LANDING OR THE LIKE; OR (B) 865mm ABOVE THE FLOOR OF A LANDING TO A STAIR WHERE THE BARRIER IS PROVIDED ALONG THE INSIDE EDGE OF THE LANDING AND DOES NOT EXCEED A LENGTH OF 500mm (b) OPENINGS IN BARRIERS MUST NOT EXCEED 125mm (c) A BARRIER. EXCEPT A WINDOW SERVING AS A BARRIER, MUST BE DESIGNED TO TAKE LOADING FORCES IN ACCORDANCE WITH AS/NZS 1170.1 (d) FOR FLOORS MORE THAN 4m ABOVE THE SURFACE BENEATH, ANY HORIZONTAL ELEMENTS WITHIN THE BARRIER BETWEEN 150mm AND 760mm ABOVE THE FLOOR MUST NOT FACILITATE CLIMBING. (f) WHERE WIRE BALUSTRAUDING IS USED IT MUST COMPLY WITH PART H5D3.
- (a) HANDRAILS TO A STAIRWAY OR RAMP MUST -(i) BE LOCATED ALONG AT LEAST 1 SIDE OF THE FLIGHT OR RAMP; AND (ii) BE LOCATED ALONG THE FULL LENGTH OF THE FLIGHT OR RAMP, EXCEPT WHERE IT MEETS A BARRIER (iii) HAVE A TOP SURFACE HANDRAIL HEIGHT OF NOT LESS THAN 865mm VERTICALLY ABOVE THE NOSING OF THE STAIRS.
- (a) A WINDOW OPENING MUST BE PROVIDED WITH PROTECTION, IF THE FLOOR BELOW THE WINDOW IN A BEDROOM IS 2m OR MORE ABOVE THE SURFACE BENEATH. (b) WHERE THE LOWEST LEVEL OF THE WINDOW OPENING IS LESS THAN 1.7m ABOVE THE FLOOR, A WINDOW OPENING COVERED BY (a) MUST COMPLY WITH THE FOLLOWING: (i) THE OPENABLE PORTION OF THE WINDOW MUST BE PROTECTED WITH -(A) A DEVICE CAPABLE OF RESTRICTING THE WINDOW OPENINGS; OR (B) A SCREEN WITH SECURE FITTINGS (ii) A DEVISE OR SCREEN REQUIRED BY (i) NUST -(A) NOT PERMIT A 125mm SPHERE TO PASS THROUGH THE WINDOW OPENING OR SCREEN; AND (B) RESIST AN OUTWARD HORIZONTAL ACTION OF 250N AGAINST THE -(aa) WINDOW RESTRAINED BY A DEVICE; OR (bb) SCREEN PROTECTING THE OPENING; AND (C) HAVE A CHILD RESISTANT RELEASE MECHANISM IF THE SCREEN OR DEVICE IS ABLE TO BE REMOVED. UNLOCKED OR OVERRIDDEN.

ROOM HEIGHTS:

- 1. IN ACCORDANCE WITH PART H4D4 OF CURRENT N.C.C.
- 2. CEILING HEIGHTS MUST NOT BE LESS THAN: (a) IN A HABITABLE ROOM EXCLUDING A KITCHEN - 2.4m (b) IN A KITCHEN - 2.1m (c) IN A CORRIDOR, PASSAGEWAY OR THE LIKE - 2.1m (d) IN A BATHROOM, LAUNDRRY, WC, PANTRY, STOREROOM, GARAGE OR THE LIKE -2.1m (e) IN A ROOM WITH A SLOPING CEILING OR PROJECTIONS BELOW THE CEILING LINE WITHIN: (i) A HABITABLE ROOM -(A) IN AN ATTIC - NOT LESS THAN 2.2m FOR AT LEAST 2/3 OF THE FLOOR AREA OF THE ROOM (B) I OTHER ROOMS - NOT LESS THAN 2.4m OVER 2/3 OF THE FLOOR AREA OF THE ROOM (ii) A NON-HABITABLE ROOM - NOT LESS THAN 2.1m FOR AT LEAST 2/3 OF THE ROOM AREA (f) IN A STAIRWAY, RAMP OR LANDING - 2.0m MEASURE VERTICALLY ABOVE THE NOSING OR SURFACE.

AIR MOVEMENT:

1. IN ACCORDANCE WITH PART H4V3 OF CURRENT N.C.C.

BUSHFIRE PRONE AREAS:

- 1. IN ACCORDANCE WITH PART H7D4 OF CURRENT N.C.C AND AS3959-2009.
- AN ASSESSMENT TO DETERMINE THE BAL RATING MUST BE UNDERTAKEN AND ANY REQUIREMENTS FROM THE SUBSEQUENT BALLEVEL MUST BE APPLIED.
- 3. A CLASS 1 BUILDING WITHIN A DESIGNATED BUSHFIRE PRONE AREA MUST BE IN ACCORDANCE WITH TAS 3.7.4.1 VEHICULAR ACCESS AND TAS 3.7.4.2 WATER SUPPLY.

VENTILATION:

- IN ACCORDANCE WITH PART H4P5 OF CURRENT N.C.C.
- AN EXHAUST FAN OR OTHERS MEANS OF MECHANICAL VENTILATION MAY BE USED TO VENTILATE A SANITARY COMPARTMENT, LAUNDRY OR BATHROOM PROVIDED CONTAMINATED AIR EXHAUSTS-(i) DIRECTLY OUTSIDE THE BUILDING BY WAY OF DUCTS; OR (ii) INTO A ROOF SPACE-(A) IS ADEQUATLY VENTILATED BY OPEN EAVES, AND/OR ROOF VENTS (B) IS COVERED BY ROOF TILES WITHOUT SARKING.
- SANITARY COMPARTMENTS MUST NOT OPEN DIRECTLY INTO A KITCHEN OR PANTRY UNLESS-(a) ACCESS IS BY AN AIRLOCK, HALLWAY OR OTHER ROOM (b) THE ROOM CONTAINING THE CLOSET PAN IS PROVIDED WITH MECHANICAL EXHAUST.

SOUND INSULATION:

1. IN ACCORDANCE WITH PART H4P6 OF CURRENT N.C.C.

FACILITIES:

- IN ACCORDANCE WITH PART H4D5 OF CURRENT N.C.C
- THE DOOR TO A SANITARY CLOSED COMPARTMENT MUST -(a) OPEN OUTWARDS OR (b) SLIDE OR (c) BE READILY REMOVABLE FROM THE OUTSIDE OF THE COMPARTMENT UNLESS THERE IS A CLEAR SPACE OF 1.2m BETWEEN THE PAN AND THE DOORWAY

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REV DATE REMARK

P1 26.09.2024 PRELIMINARY ISSUE

LYNE DESIGN

BUILDING DESIGN/ DRAFTING - BUSHFIRE MANAGEMENT DAVID LYNE ACCREDITED DESIGNER: CC7063 11 GRANVILLE AVENUE GEILSTON BAY TASMANIA 7015

MOBILE: 0421 852 987 dave lyne@hotmail.com

28 RIVIERA DRIVE CARLTON, TAS 7173

NOTES SHEET

Accepted (Client 1)	OLIVIA MARANESI	Date
Accepted (Client 2)	DANIEL WOODWARD	Date
Approved (Builder)	NOT APPLICABLE	Date

This document must be signed

SCALES @ A3	DESIGNED BY	DRAWN BY
N/A	D. LYNE	D. LYNE
	PLOT DATE	09/10/2024

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PLOT DETAILS RIVIERA.DWG

PROJECT NO. 1566/24

B02

REVISION P1

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Plans Reference:P1 Date Received:11/10/2024

ALL SITE WORKS SHALL BE IN ACCORDANCE WITH N.C.C., CSIRO BTF 18,19,22 AND AS2870 MINIMAL SITE DISTURBANCE IS TO BE CARRIED OUT. SEDIMENT CONTROL; 'GEOLAB' SILT FENCE 1000 OR SIMILAR. TOPSOIL STOCKPILES REMAINING ON THE SITE TO BE COVERED WITH PLASTIC, ADEQUATELY RETAINED ALONG ALL EDGES. UNUSED STOCKPILES TO BE REMOVED FROM SITE OR USED FOR FUTURE LANDSCAPING.

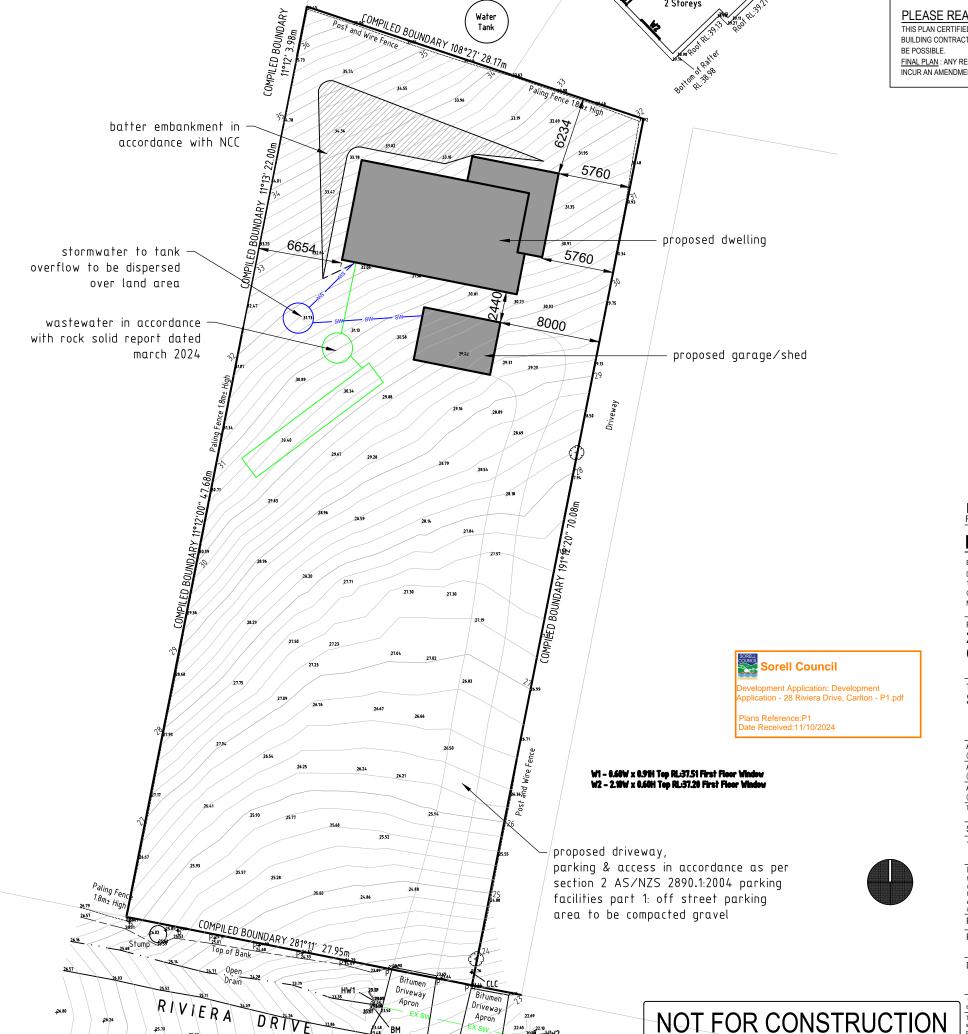
SITE PREPARATION AND EXCAVATION

IN ACCORDANCE WITH PART 3.1 OF CURRENT N.C.C., AND TO LOCAL COUNCIL REQUIREMENTS.

INTERNAL FINISHED FLOOR LEVEL (FFL) TO BE MIN. 150mm ABOVE FINISHED EXTERNAL GROUND AREAS (FLOWER BEDS OR GRASSED AREAS) AND MIN. 50mm ABOVE FINISHED EXTERNAL SEALED SURFACES (PAVED AREAS). PROVIDE 50mm MIN. FALL FOR THE FIRST METER AWAY FROM BUILDING TOWARDS LOWER GROUND OR ALTERNATIVELY SUFFICIENT DRAINAGE PROVISIONS (AG DRAINS, SUMPS OR SIMILAR).

SITE PLAN

SCALE: 1:300



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P1 26.09.2024 PRELIMINARY ISSUE REV DATE REMARK

LYNE DESIGN

BUILDING DESIGN/ DRAFTING - BUSHFIRE MANAGEMENT DAVID LYNE 11 GRANVILLE AVENUE ACCREDITED DESIGNER: CC7063 GEILSTON BAY, TASMANIA 7015

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28 RIVIERA DRIVE CARLTON, TAS 7173

SITE PLAN

Accepted (Client 1)	OLIVIA MARANESI	Date
Accepted (Client 2)	DANIEL WOODWARD	Date
Approved (Builder)	NOT APPLICABLE	Date

	PLOT DATE	10/10/2024
1:300	D.LYNE	D.LYNE
SCALES @ A3	DESIGNED BY	I DRAWN BY

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PLOT DETAILS RIVIERA.DWG

PROJECT NO. 1566/24

DWG NO. REVISION P1 B03

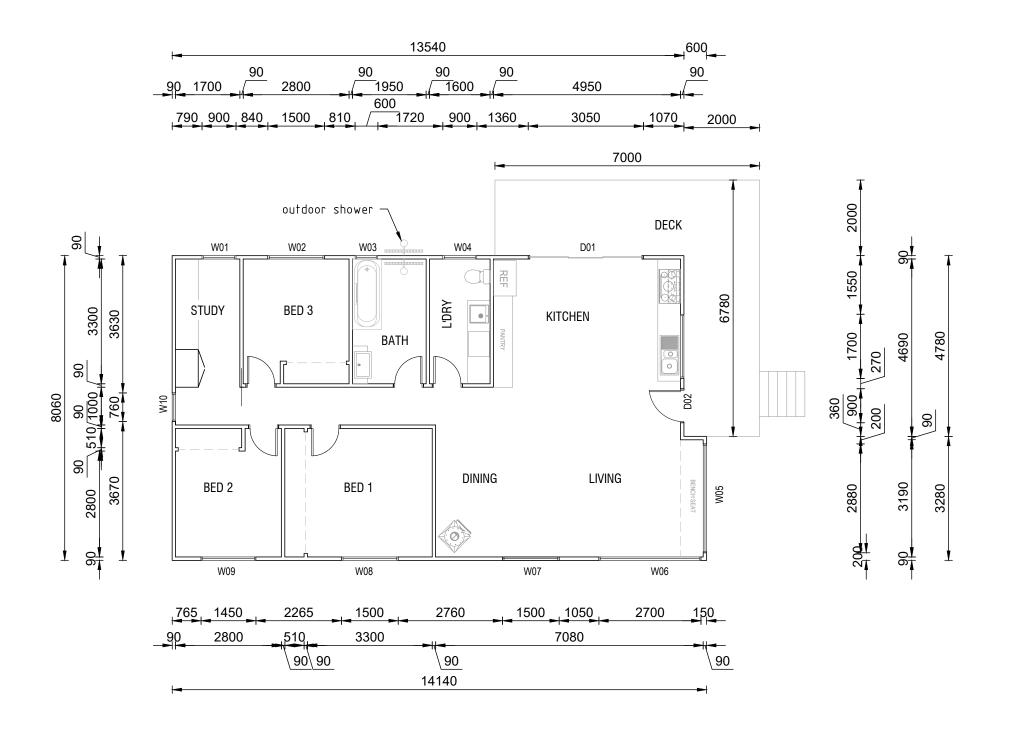
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FLOOR PLAN

SCALE: 1:100





P1 26.09.2024 PRELIMINARY ISSUE REV DATE REMARK

LYNE DESIGN

BUILDING DESIGN/ DRAFTING - BUSHFIRE MANAGEMENT DAVID LYNE 11 GRANVILLE AVENUE ACCREDITED DESIGNER: CC7063 GEILSTON BAY, TASMANIA 7015

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28 RIVIERA DRIVE CARLTON, TAS 7173

FLOOR PLAN

Accepted (Client 1)	OLIVIA MARANESI	Date
Accepted (Client 2)	DANIEL WOODWARD	Date
Approved (Builder)	NOT APPLICABLE	Date

This document must be signed

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1:100	D.LYNE	D.LYNE
	PLOT DATE	09/10/2024

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PLOT DETAILS RIVIERA.DWG

PROJECT NO. 1566/24

REVISION P1 B04

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PLEASE READ CAREFULLY THIS PLAN CERTIFIED CORRECT IS THE ONE REFERRED TO IN THE BUILDING CONTRACT AND I UNDERSTAND CHANGES HEREAFTER MAY NOT BE POSSIBLE. FINAL PLAN: ANY REQUESTED VARIATIONS TO YOUR HOUSE PLAN WILL INCUR AN AMENDMENT / ADMINISTRATION MINIMUM FEE colorbond custom orb roof cladding 13° pitch – shale grey colorbond custom orb cladding shiplap timber cladding shale grey colorbond custom orb roof cladding 9° pitch - shale grey CL 34.9 FL 32.5 select facebrick - TBA **EASTERN ELEVATION SOUTHERN ELEVATION** SCALE: 1:100 SCALE: 1:100 P1 26.09.2024 PRELIMINARY ISSUE REV DATE REMARK LYNE DESIGN BUILDING DESIGN/ DRAFTING - BUSHFIRE MANAGEMENT colorbond custom orb cladding colorbond custom orb cladding DAVID LYNE 11 GRANVILLE AVENUE ACCREDITED DESIGNER: CC7063 shale grey GEILSTON BAY, TASMANIA 7015 MOBILE: 0421 852 987 dave_lyne@hotmail.com shale grey PROJECT 28 RIVIERA DRIVE CARLTON, TAS 7173 CL 34.9 **ELEVATIONS SHEET** FL 32.50 Accepted OLIVIA MARANESI (Client 1) Accepted DANIEL WOODWARD Date (Client 2) aluminum framed doors & windows Approved NOT APPLICABLE Date - black WESTERN ELEVATION NORTHERN ELEVATION SCALES @ A3 DESIGNED BY DRAWN BY SCALE: 1:100 SCALE: 1:100 D.LYNE PLOT DATE 09/10/2024 DO NOT SCALE. Use only figured dimensions. Locations of structure, fittings, services etc on this drawing are indicative only. CONTRACTOR to check all other project drawings for co-ordination between structure, fabric, fixtures, fittings, services etc. CONTRACTOR to site check all dimensions and exact locations of all items. no responsibility shall be taken for dimensional information scaled or digitally derived from this document. PLOT DETAILS RIVIERA.DWG PROJECT NO. 1566/24 Sorell Council SCALE 1:100 evelopment Application: Development pplication - 28 Riviera Drive, Carlton - P1.pdf DWG NO. REVISION

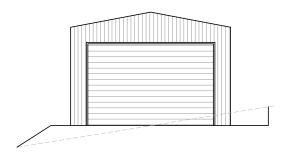
Plans Reference:P1 Date Received:11/10/2024

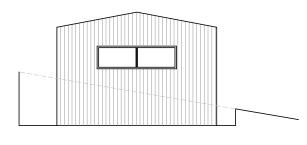


P1

B05

4780 900_500 2100 6180 **FLOOR PLAN**





SOUTHERN ELEVATION

SCALE: 1:100

SCALE: 1:100

EASTERN ELEVATION SCALE: 1:100

WESTERN ELEVATION

SCALE: 1:100

P1 26.09.2024 PRELIMINARY ISSUE REV DATE REMARK

LYNE DESIGN

BUILDING DESIGN/ DRAFTING - BUSHFIRE MANAGEMENT DAVID LYNE 11 GRANVILLE AVENUE ACCREDITED DESIGNER: CC7063 GEILSTON BAY, TASMANIA 7015
MOBILE: 0421 852 987 dave_lyne@hotmail.com

PROJECT

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28 RIVIERA DRIVE CARLTON, TAS 7173

ROOF PLAN

Accepted (Client 1)	OLIVIA MARANESI	Date
Accepted (Client 2)	DANIEL WOODWARD	Date
Approved (Builder)	NOT APPLICABLE	Date

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1:100	D.LYNE	D.LYNE
	PLOT DATE	09/10/2024

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PROJECT NO. 1566/24

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NORTHERN ELEVATION

SCALE: 1:100

SHED PLAN - FLOOR & ELEVATIONS

SCALE: 1:100

